Drainage Report:

42 Park Street
Ayer, MA

Submitted to:

Town of Ayer
Planning Board

September 1, 2023

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Mixed-Use Development
42 Park Street
Ayer, Massachusetts
STORM WATER MANAGEMENT DESIGN
September 1, 2023

INTRODUCTION

The project site is approximately 0.48 acres in size and is located in the General Business District at 42 Park Street. The site is partially developed and contains a multi-family home that is currently vacant with a asphalt driveway. The site is bounded by Park Street (Route 111) to the east, a commercial property which is the site of Jiffy Lube to the south, the Boston and Maine Railroad to the west, and commercial property to the north. Refer to **Figure 1** for the Locus Plan.

The proposed project includes the renovation of the existing multi-family structure and construction of an addition to the existing structure with associated site improvements. These site improvements include the construction of 12 parking spaces, installation of new drainage improvements, utility connections, site grading, and the installation of new lighting and landscaping. Proposed stormwater runoff with be collected by catch basins and be conveyed through a water quality unit to the proposed infiltration basin located on the northwestern portion of the property.

The hydrologic study area is comprised of approximately 0.51 acres. Based on the USDA Natural Resources Conservation Service soil survey the site is comprised of a mix of Merimac-Urban Land Complex, which is a Hydrologic Soil Group (HSG) "A" Soil, and Urban land. Based on the soil survey the proposed infiltration basin is located within the HSG "A" Soil, thus an infiltration rate of 2.41 was used in the drainage analysis. Refer to **Appendix A** for the NRCS soil survey.

EXISTING CONDITIONS

As described above, the existing site is partially developed and is comprised of approximately 0.34 acres of grass, 0.08 acres of pavement, 0.04 acres of rooftop, and 0.05 acres of wood. The existing site is made up of one watershed area.

Area 1 includes the entire hydrologic study area. This area sheet flows from the eastern side of the property across to the western side of the property, which is considered Point of Analysis 1 (POA-1). Refer to **Figure 2**– Existing Watershed Plan.

PROPOSED CONDITIONS

Under proposed conditions, the site is comprised of approximately 0.21 acres of grass, 0.03 acres of concrete, 0.15 acres of pavement, 0.08 acres of rooftop, and 0.04 acres of woods. The post-development run-off rates will be mitigated to less than the pre-development run-off rates for all design storm events. The proposed site is comprised of 3 watershed areas.

Area 1 includes the entire rooftop area of the proposed building. This area is collected through the roof leader system and is conveyed to the proposed infiltration basin. The infiltration basin ultimately discharges to the western side of the property. This area is considered Point of Analysis 1 (POA-1).

Area 2 includes the proposed developed area including the drive aisles, parking fields, and proposed infiltration basin. This area sheet flows to the proposed catch basins and is conveyed through a water quality unit to the proposed infiltration basin, which ultimately discharges to POA-1.

Area 3 includes the remaining area that is located downslope of the proposed development. This area sheets off to POA-1.Refer to **Figure 3**– Proposed Watershed Plan.

STORMWATER MANAGEMENT

The proposed drainage design was based on the Massachusetts Department of Environmental Protection (MADEP) Stormwater Management Standards (Stormwater Policy, latest edition). The standards have been revised to promote increased stormwater recharge, the treatment of more runoff from polluting land uses, pollution prevention, the removal of illicit discharges to the stormwater management systems, and improved operation and maintenance of stormwater best management practices (BMP's). In addition to the MADEP Policy, the project was designed to meet the Town of Ayer Stormwater Bylaws which require that the project provide 90% TSS removal and 60% Phosphorus removal for new development in addition to meeting the 10 MADEP Stormwater Standards. The following summarizes the proposed project's compliance with both the MADEP Stormwater Management Standards and the City of Leominster Stormwater Bylaws.

Standard #1 Untreated Storm Water: No new untreated storm water conveyances have been proposed to discharge untreated stormwater directly to or cause erosion in wetlands or waters of the Commonwealth. Storm water Best Management Practices (BMP's), such as deep sump catch basins, water quality unit and at grade infiltration basin are proposed within the project to provide stormwater quality control prior to discharging runoff from the site.

Standard #2 Post-Development Peak Discharge Rates: BMP's have been developed to attenuate the peak discharge rates for the 2, 10, 25, and 100 year, 24-hour storm events. Refer to **Table 1** for the pre- and post-development peak discharge rates.

Standard #3 Recharge to Groundwater: The project site is located within Hydrologic Soil Group (HSG) "A" classified soils. Per MA DEP standards, recharge is required to eliminate or minimize the loss of annual recharge to groundwater using environmentally sensitive site design, BMP's and good operation and maintenance. The required recharge volume for the proposed project is 0.013 AC-FT. For the 2-year storm event, the at grade infiltration basin will provide approximately 0.049 AC-FT of recharge to groundwater. Refer to the HydroCAD model in **Appendix B**. In addition, the project will provide approximately 1,484 CF of volume (>1" water quality volume) below the outlet to the infiltration basin. Because the at grade basin is located within an "A" soil, the water quality volume will draw down within 25 hours after the end of the storm event. Refer to **Appendix A** for the recharge calculations and drawdown analysis.

Standard #4 80 Percent TSS Removal: Based on the proposed stormwater management system design, the proposed BMP's will remove more than 90% of the Total Suspended Solids (TSS) and 60% of Phosphorus from the stormwater runoff discharging from the site to meet compliance with the Town of Ayer Stormwater Bylaws. This is being achieved through the use of a water quality inlet units, and an at below grade infiltration basin prior to infiltrating. These BMP's are sized to capture in excess of the required water quality volume. As noted above, the proposed infiltration basin will treat the required water quality volume, which will be provided below the outlets in the basin. Refer to **Appendix A** for the Water Quality Volume calculations and the TSS Removal Worksheet.

Standard #5 Higher Potential Pollutant Loads: The proposed project is not classified by the DEP as a source for higher pollutant loads.

Standard #6 Protection of Critical Areas: The project site is not considered a critical area as defined by the MA DEP.

Standard #7 Redevelopment Project: The project is not considered a redevelopment site.

Standard #8 Erosion/Sediment Control: Erosion and sediment controls are incorporated into the project design to prevent erosion, control sediment movement, and stabilize exposed soils during construction. During construction, control practices will be utilized such as the placement of straw wattles, silt fencing, and the implementation of soil stabilization practices. These control measures will be periodically checked and maintained as necessary throughout the entire construction duration.

Standard #9 Operation/Maintenance Plan: A long term operation and maintenance plan has been developed to ensure the stormwater management system will function as designed. See **Appendix A** for the Operation and Maintenance Plan.

Standard #10 Illicit Discharges to Stormwater Management System: The Stormwater Management System associated with the development has been designed such that prior to storm water runoff discharging from the site, it is treated through a series of best management practices. To the Engineer's knowledge, there are no known or designed non-storm water discharges that are or will be connected to the storm water collection system that would convey pollutants directly to groundwater or surface waters. Refer to **Appendix A** for the Illicit Discharge Compliance Statement.

The proposed design meets **all** applicable DEP Stormwater Management Standards and the Town of Grafton Stormwater Bylaws. Refer to **Appendix A** for the MADEP Stormwater Checklist.

DRAINAGE COLLECTION SYSTEM DESIGN

The proposed drain pipe network is composed of deep sump catch basin and manholes that will collect runoff from the parking and landscaped areas within the proposed development and convey it to the proposed infiltration basin. The pipe layout is depicted on the Grading and Drainage Plan in the plan set.

Pipe sizes were determined using the Rational Method to determine contributing flows to catch basin, as well as the Manning's Equation to calculate pipe flows (refer to **Appendix A** for pipe sizing calculations.)

The following criteria were used to design the pipe network:

- Manholes are provided at all changes in direction or changes in pipe size.
- Pipe sizes are based on flows for the 25-year storm frequency.
- Storm drain pipes shall be HDPE unless otherwise noted.
- Pipe flow velocities are maintained at a maximum of 12 fps.

STORMWATER QUANTITY

Due to the proposed increase in impervious area, the project will require BMP's for infiltration and detention in order to comply with Standards # 2 and #3 of the DEP Stormwater Management Policy. The stormwater facilities proposed will an at grade infiltration basins. The proposed basin will recharge the required water quality volume in

addition to attenuating the peak runoff rates for the 2, 10, 25 and 100-year, 24-hour storm events.

Hydrologic analyses were performed utilizing the computer program, HydroCAD©. In order to determine the peak rate of discharge for existing and proposed conditions, runoff hydrographs were generated for the 2, 10, 25, and 100-year, 24-hour storm events using the SCS TR-20 Method and Type III rainfall distribution. Precipitation amounts utilized in the analysis are as defined by NRCC Extreme Precipitation Data (refer to **Appendix A** for the NRCC Precipitation Tables and **Appendix B** for the existing and proposed HydroCAD models). Under proposed conditions, the post development runoff hydrographs were flood routed through the proposed stormwater management facilities.

Table 1 compares peak runoff rates for the 2-, 10-, 25-, and 100-year storm events for existing and proposed conditions.

Table 1 Comparison of Peak Runoff Rates

Storm Event	Existing Flow (cfs)	Proposed Flow (cfs)
	POA-1	POA-1
2-Year	0.01	0.00
10-Year	0.16	0.00
25-Year	0.42	0.28
100-Year	1.15	0.89

As shown in Table 1, peak runoff rates under proposed conditions are less than existing conditions for the 2-, 10-, 25- and 100-year storm events. Therefore, the proposed stormwater design complies with Standard #2 of the MA DEP Stormwater Management Policy.

STORMWATER QUALITY

All stormwater runoff will be treated to address water quality concerns through the use of DEP approved BMP's. The following BMP's will be provided on-site and when combined will achieve an excess of 90% TSS removal: deep sump hooded catch basins, water quality unit and an at grade infiltration basin. (See **Appendix A** for TSS Removal Worksheets

Water Quality Units

The proposed design of the on-site drainage system will incorporate a Hydroworks water quality unit prior to connecting to the at grade infiltration basin on site. Maintenance will be performed per the manufacturer's recommendations; however basic maintenance will consist of monthly inspections and after each major storm event during the first year of installation to accurately establish the required maintenance schedule. The structures will be cleaned out twice per year or upon the stored volume reaching 15% of the particle separator's capacity, or immediately in the event of a spill.

Below Grade Infiltration Basin

Once constructed, infiltration basin will be inspected at a minimum after several storm events for the first year and annually thereafter to confirm drainage system functions as designed. Problems will be addressed immediately. System shall be cleaned as required per the manufacturer's recommendations.

Phosphorus Removal

The 60% phosphorus removal requirement has been achieved through the use of stormwater BMP's that are documented within Volume 2 of the Massachusetts Stormwater Handbook. The proposed water quality unit has been sized to treat the 1" Water Quality Volume, and the at grade infiltration basin has been sized to provided greater than the 1" WQV.

CONCLUSION

The proposed stormwater management plan for the project addresses both water quantity and quality issues and conforms to the standards outlined in the revised MADEP Stormwater Management Policy.

Figures

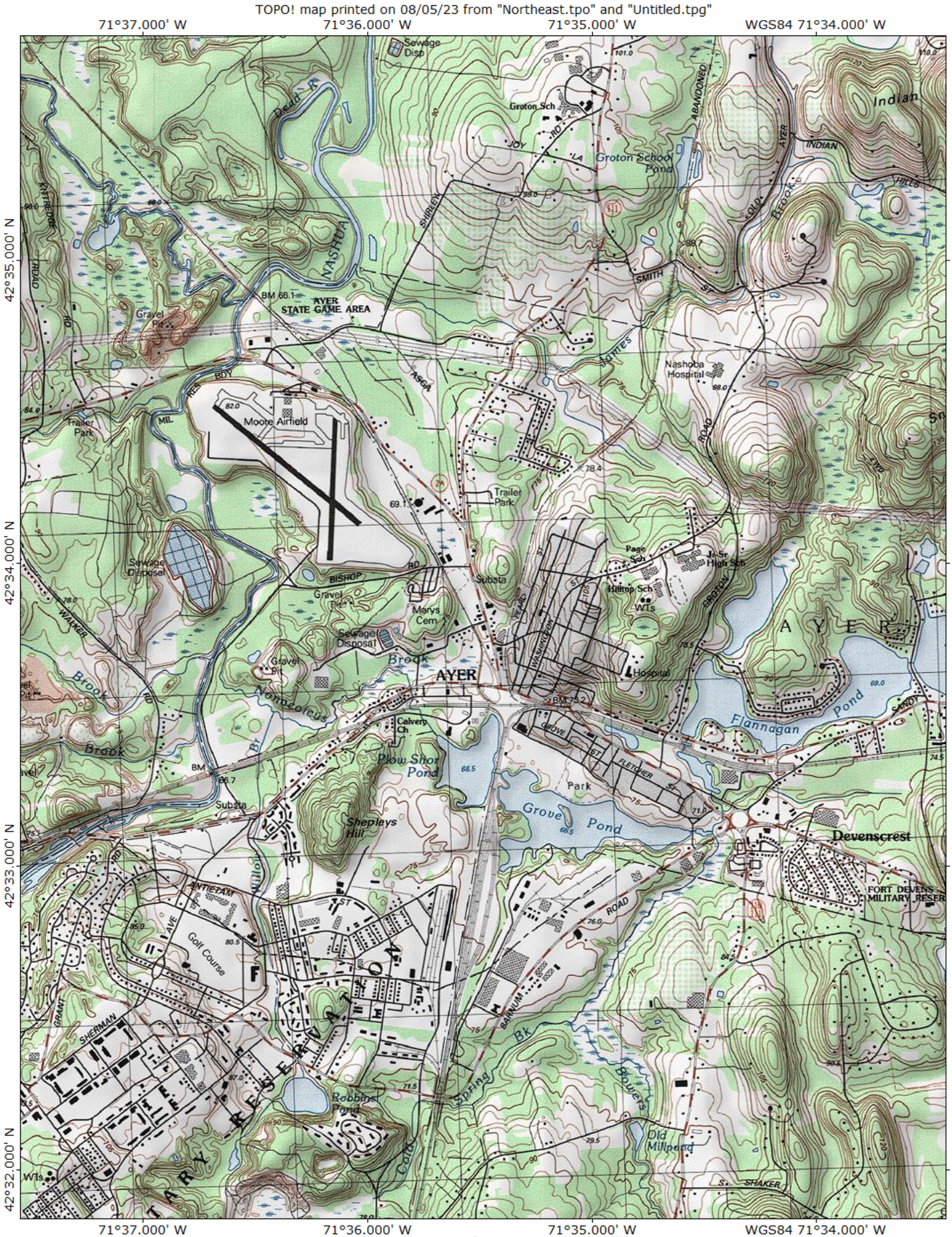


Figure 1
Site Locus

A.P. 26-15
N/F
MANNONE
BK. 65321 PG. 496

SEE PLAN NO. 935 OF 2005

BOSTON & MAINE RAILROAD
(FITCHBURG DIVISION - GREENVILLE BRANCH)

1R

SEWER EASEMENT
BK. 14336 PG. 138

20' SETBACK LINE

25' SETBACK LINE

30' SETBACK LINE

EOP

SMH
R=216.30
I=212.88

6" S

20" DEAD

12" MAPLE X3

18" MAPLE X3

10" MAPLE

8" MAPLE

8" MAPLE

#44

EXISTING DWELLING
FFE=224.59
#42

TR

W

OHW

GC

ASPHALT DRIVEWAY

ACCESS

PARK STREET (RTE 2A & 111)
(PUBLIC - 1898 TOWN LAYOUT 50' WIDE)

SB/DH (F)

RIGHT-OF-WAY LIMIT

SITE BENCHMARK
MAG NAIL
ELEV.=224.53

BIT. CONC. WALK

GRAVEL WALL W/ 4' CLR.

AREA 1
TOTAL=22,063 SF
ROOFTOP "A"=1,515 SF
PVMNT "A"=3,365 SF
GRASS "A"=14,861 SF
WOODS "A"=2,322 SF

A.P. 26-18
N/F
NATIONAL RETAIL PROPERTIES LP
BK. 64363 PG. 575

No.	Date	Revision

Drawn By: JLL
Designed By: JLL
Checked By:

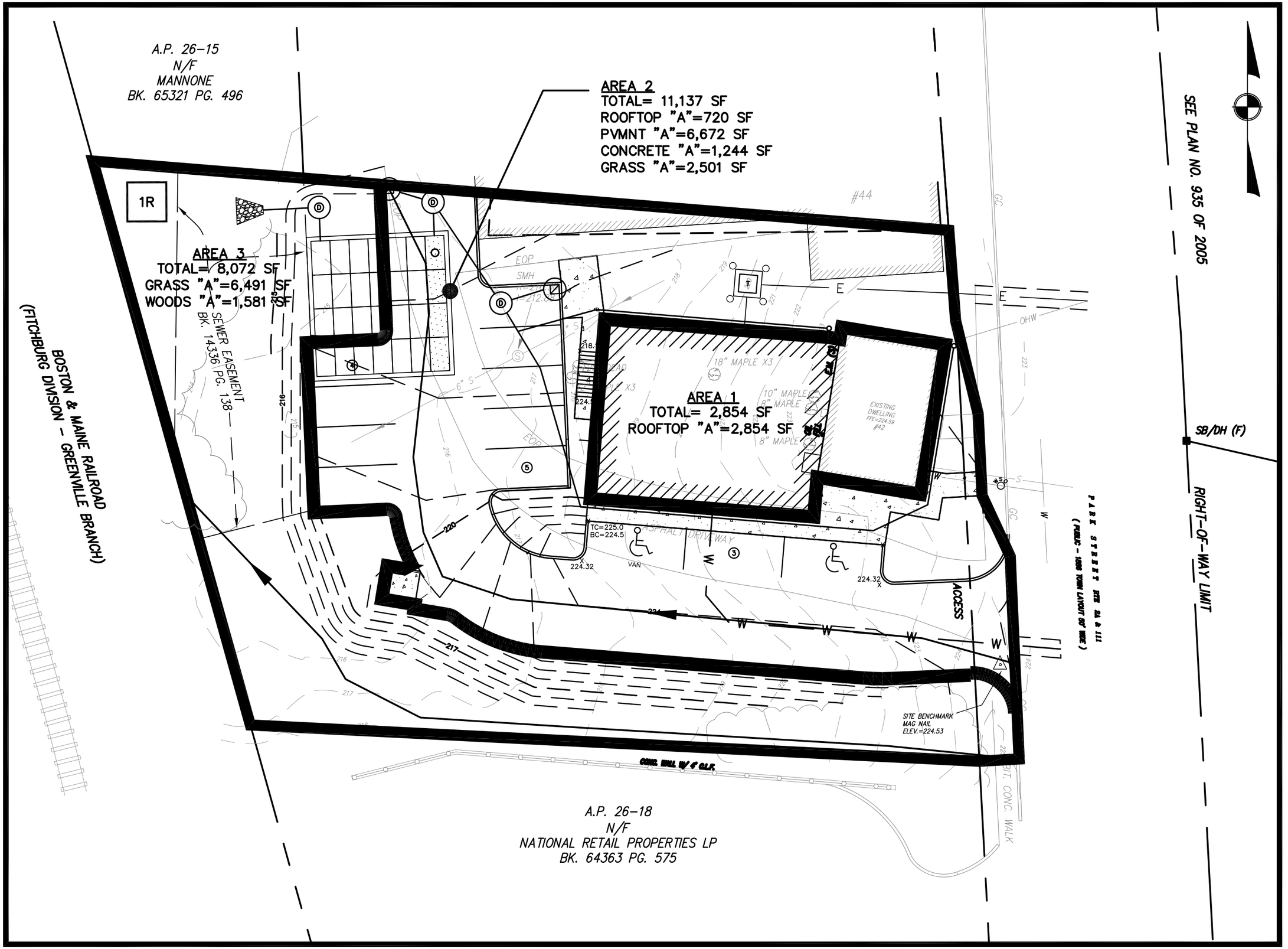
McCarty Engineering, Inc.
Civil Engineers
42 Tucker Drive, Leominster, MA 01453
phone:(978) 534-1318 fax: (978) 840-6907

Project Name
Proposed Mixed-Use Development
42 Park Street
Ayer, MA 01432

Sheet Title
Existing Watershed Plan

Job No: Park Street
File Name: Existing Drainage
Date: 9/1/2023
Scale: 1"=20'

Sheet No.
2



No.	Date	Revision

Drawn By: JLL | Designed By: JLL | Checked By:

McCarty Engineering, Inc.
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42 Tucker Drive, Leominster, MA 01453
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Project Name

Proposed Mixed-Use Development
42 Park Street
Ayer, MA 01432

Sheet Title

Proposed Watershed Plan

Job No: Park Street
File Name: Proposed Drainage
Date: 9/1/2023
Scale: 1"=20'

Sheet No. 3

Appendix A

NRCS Soil Survey

Map Scale: 1:666 if printed on A landscape (11" x 8.5") sheet.

N

Meters
0 5 10 20 30

Feet
0 30 60 120 180

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 19N WGS84

MAP LEGEND

Area of Interest (AOI)

Area of Interest (AOI)

Soils

Soil Rating Polygons

A
A/D
B
B/D
C
C/D
D
Not rated or not available

Soil Rating Lines

A
A/D
B
B/D
C
C/D
D
Not rated or not available

Soil Rating Points

A
A/D
B
B/D
C
C/D
D
Not rated or not available

Water Features

Streams and Canals

Transportation

Rails
Interstate Highways
US Routes
Major Roads
Local Roads

Background

Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:25,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Middlesex County, Massachusetts
Survey Area Data: Version 22, Sep 9, 2022

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: May 22, 2022—Jun 5, 2022

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
602	Urban land		0.7	57.4%
626B	Merrimac-Urban land complex, 0 to 8 percent slopes	A	0.5	42.6%
Totals for Area of Interest			**1.2**	**100.0%**

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher

Recharge Calculations

McCarty Engineering, INC. Project: 42 Park Street
Stormwater Recharge

Date: 9/1/23
Comp: JLL
City: Ayer Check : BRM
State: MA

Recharge Required

Hydrologic Soil Goup	Volume to Recharge (in)
A	**0.6**

Required Recharge Volume

Soil group	Impervious Area (ac)	Required Volume (ac-ft)
A	0.26	0.013
	Total	0.013

Recharge Provided

***Total Recharge Provided in Infiltration Basin during the 2-year storm= 0.049 AC-FT**

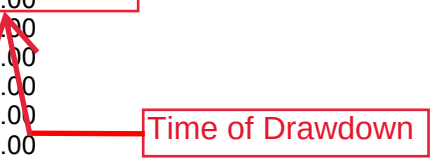
**All recharge is taking place through the bottom of the infiltration basins. Refer to the Proposed Conditions HydroCAD Model for the Recharge Volume for all storm events.*

Drawdown Analysis

Drawdown Analysis-Infiltration Basin

Hydrograph for Pond 1P: Underground Basin (continued)

Time (hours)	Inflow (cfs)	Storage (cubic-feet)	Elevation (feet)	Outflow (cfs)	Discarded (cfs)	Primary (cfs)
23.85	0.02	6	212.01	0.02	0.02	0.00
23.90	0.02	6	212.01	0.02	0.02	0.00
23.95	0.02	5	212.01	0.02	0.02	0.00
24.00	0.02	5	212.01	0.02	0.02	0.00
24.05	0.02	5	212.01	0.02	0.02	0.00
24.10	0.00	4	212.01	0.02	0.02	0.00
24.15	0.00	2	212.00	0.01	0.01	0.00
24.20	0.00	1	212.00	0.00	0.00	0.00
24.25	0.00	1	212.00	0.00	0.00	0.00
24.30	0.00	0	212.00	0.00	0.00	0.00
24.35	0.00	0	212.00	0.00	0.00	0.00
24.40	0.00	0	212.00	0.00	0.00	0.00
24.45	0.00	0	212.00	0.00	0.00	0.00
24.50	0.00	0	212.00	0.00	0.00	0.00
24.55	0.00	0	212.00	0.00	0.00	0.00
24.60	0.00	0	212.00	0.00	0.00	0.00
24.65	0.00	0	212.00	0.00	0.00	0.00
24.70	0.00	0	212.00	0.00	0.00	0.00
24.75	0.00	0	212.00	0.00	0.00	0.00
24.80	0.00	0	212.00	0.00	0.00	0.00
24.85	0.00	0	212.00	0.00	0.00	0.00
24.90	0.00	0	212.00	0.00	0.00	0.00
24.95	0.00	0	212.00	0.00	0.00	0.00
25.00	0.00	0	212.00	0.00	0.00	0.00



Water Quality Volume & Flow Rate Calculations

Water Quality Volume Calculation-Infiltration Basin

Stage-Area-Storage for Pond 1P: Underground Basin (continued)

Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)
213.06	1,115	716	213.59	1,115	1,161
213.07	1,115	725	213.60	1,115	1,169
213.08	1,115	733	213.61	1,115	1,177
213.09	1,115	742	213.62	1,115	1,186
213.10	1,115	750	213.63	1,115	1,194
213.11	1,115	759	213.64	1,115	1,202
213.12	1,115	768	213.65	1,115	1,210
213.13	1,115	776	213.66	1,115	1,218
213.14	1,115	785	213.67	1,115	1,226
213.15	1,115	793	213.68	1,115	1,234
213.16	1,115	802	213.69	1,115	1,242
213.17	1,115	810	213.70	1,115	1,250
213.18	1,115	819	213.71	1,115	1,258
213.19	1,115	827	213.72	1,115	1,266
213.20	1,115	836	213.73	1,115	1,274
213.21	1,115	844	213.74	1,115	1,282
213.22	1,115	853	213.75	1,115	1,290
213.23	1,115	861	213.76	1,115	1,298
213.24	1,115	870	213.77	1,115	1,306
213.25	1,115	878	213.78	1,115	1,314
213.26	1,115	887	213.79	1,115	1,322
213.27	1,115	895	213.80	1,115	1,329
213.28	1,115	904	213.81	1,115	1,337
213.29	1,115	912	213.82	1,115	1,345
213.30	1,115	921	213.83	1,115	1,353
213.31	1,115	929	213.84	1,115	1,361
213.32	1,115	938	213.85	1,115	1,369
213.33	1,115	946	213.86	1,115	1,376
213.34	1,115	954	213.87	1,115	1,384
213.35	1,115	963	213.88	1,115	1,392
213.36	1,115	971	213.89	1,115	1,400
213.37	1,115	980	213.90	1,115	1,408
213.38	1,115	988	213.91	1,115	1,415
213.39	1,115	996	213.92	1,115	1,423
213.40	1,115	1,005	213.93	1,115	1,431
213.41	1,115	1,013	213.94	1,115	1,438
213.42	1,115	1,021	213.95	1,115	1,446
213.43	1,115	1,030	213.96	1,115	1,454
213.44	1,115	1,038	213.97	1,115	1,461
213.45	1,115		213.98	1,115	1,469
213.46	1,115		213.99	1,115	1,477
213.47	1,115		214.00	1,115	1,484
213.48	1,115	1,071	214.01	1,115	1,492
213.49	1,115	1,079	214.02	1,115	1,499
213.50	1,115	1,087	214.03	1,115	1,507
213.51	1,115	1,096	214.04	1,115	1,514
213.52	1,115	1,104	214.05	1,115	1,522
213.53	1,115	1,112	214.06	1,115	1,529
213.54	1,115	1,120	214.07	1,115	1,537
213.55	1,115	1,129	214.08	1,115	1,544
213.56	1,115	1,137	214.09	1,115	1,552
213.57	1,115	1,145	214.10	1,115	1,559
213.58	1,115	1,153	214.11	1,115	1,567

Outlet Invert =

WQv Provided

WQv Required=1.0in x Area Inp. sf x 1ft/12in
WQv Required=1.0in x 11,490 sf x 1ft/12in =957.5 cf

1,484 cf > 957.5 cf

McCarty Engineering, INC.	Project:	42 Park Street	Proj. No:	
			Date:	9/1/23
	City:	Ayer	Comp:	JLL
	State:	MA	Check :	BRM

Converting WQv to Fow Rate for Sizing Proprietary Stormwater Treatemnnt Practices

Required WQv = 1.0 inch

$Q_{0.5}$=(qu)(A)(WQv)

qu = Unit Peak Discharge in csm/in - This Variable derived from MADEP Flow rate table, Figure 2 (atta
A = Impervious Area in square miles (sm) - 1 ac = 0.0015625 sm
WQv= Water Quality Volume in watershed inches (1.0 in)

Structure

WQU 1 (DMH 1)

Tc= 5 minutes = 0.083 hours
qu= 795 csm/in
A= 0.15 ac = 0.00024sm
WQv= 1.0 in

Flow Rate= (795 csm/in)x(0.00024sm)x(1.0 in)
Flow Rate= 0.19

McCarty Engineering, INC.	Project:	42 Park Street	Proj. No:	
			Date:	9/1/23
	City:	Ayer	Comp:	JLL
	State:	MA	Check :	BRM

Converting WQv to Fow Rate for Sizing Proprietary Stormwater Treatemnnt Practices

Required WQv = 1.0 inch

$Q_{0.5}$=(qu)(A)(WQv)

qu = Unit Peak Discharge in csm/in - This Variable derived from MADEP Flow rate table, Figure 2 (atta
A = Impervious Area in square miles (sm) - 1 ac = 0.0015625 sm
WQv= Water Quality Volume in watershed inches (1.0 in)

Structure

WQU 1 (DMH 1)

Tc= 5 minutes = 0.083 hours
qu= 795 csm/in
A= 0.05 ac = 0.00007sm
WQv= 1.0 in

Flow Rate= (795 csm/in)x(0.00007sm)x(1.0 in)
Flow Rate= 0.06

TSS Removal Worksheets

INSTRUCTIONS:
1. In BMP Column, click on Blue Cell to Activate Drop Down Menu
2. Select BMP from Drop Down Menu
3. After BMP is selected, TSS Removal and other Columns are automatically completed.

Version 1, Automated: Mar. 4, 2008

Location: WQU to Infiltration Basin

TSS Removal Calculation Worksheet

B BMP[1]	C TSS Removal Rate[1]	D Starting TSS Load*	E Amount Removed (C*D)	F Remaining Load (D-E)
Water Quality Unit	**0.99**	**1.00**	**0.99**	**0.01**
Infiltration Basin	**0.80**	**0.01**	**0.008**	**0.002**
	0.00	**0.002**	**0.00**	**0.002**
	0.00	**0.002**	**0.00**	**0.002**
	0.00	**0.002**	**0.00**	**0.002**

Total TSS Removal = **99.8%**

Separate Form Needs to be Completed for Each Outlet or BMP Train

Project: **42 Park Street**
Prepared By: **JLL**
Date: **9/1/2023**

*Equals remaining load from previous BMP (E) which enters the BMP

Non-automated TSS Calculation Sheet must be used if Proprietary BMP Proposed

1. From MassDEP Stormwater Handbook Vol. 1

INSTRUCTIONS:
1. In BMP Column, click on Blue Cell to Activate Drop Down Menu
2. Select BMP from Drop Down Menu
3. After BMP is selected, TSS Removal and other Columns are automatically completed.

Version 1, Automated: Mar. 4, 2008

Location: Infiltration Basin Pre-Treatment

TSS Removal Calculation Worksheet

B BMP[1]	C TSS Removal Rate[1]	D Starting TSS Load*	E Amount Removed (C*D)	F Remaining Load (D-E)
Water Quality Unit	0.99	1.00	0.99	0.01
	0.00	0.01	0.00	0.01
	0.00	0.01	0.00	0.01
	0.00	0.01	0.00	0.20
	0.00	0.01	0.00	0.01

Total TSS Removal = 99%

Separate Form Needs to be Completed for Each Outlet or BMP Train

Project: 42 Park Street
Prepared By: JLL
Date: 9/1/2023

*Equals remaining load from previous BMP (E) which enters the BMP

Non-automated TSS Calculation Sheet must be used if Proprietary BMP Proposed

1. From MassDEP Stormwater Handbook Vol. 1

Hydroworks Sizing Summary

42 Park St CB-1

Ayer, Ma

08-21-2023

Recommended Size: HydroDome HD 4i

A HydroDome HD 4i is recommended to provide 99 % annual TSS removal based on a drainage area of .17 (ac) with an imperviousness of 91.4 % and Sterling 2 Nnw, Massachusetts rainfall for the Hydroworks standard particle size distribution.

The recommended HydroDome HD 4i treats 100 % of the annual runoff and provides 99 % annual TSS removal for the Sterling 2 Nnw rainfall records and Hydroworks standard particle size distribution.

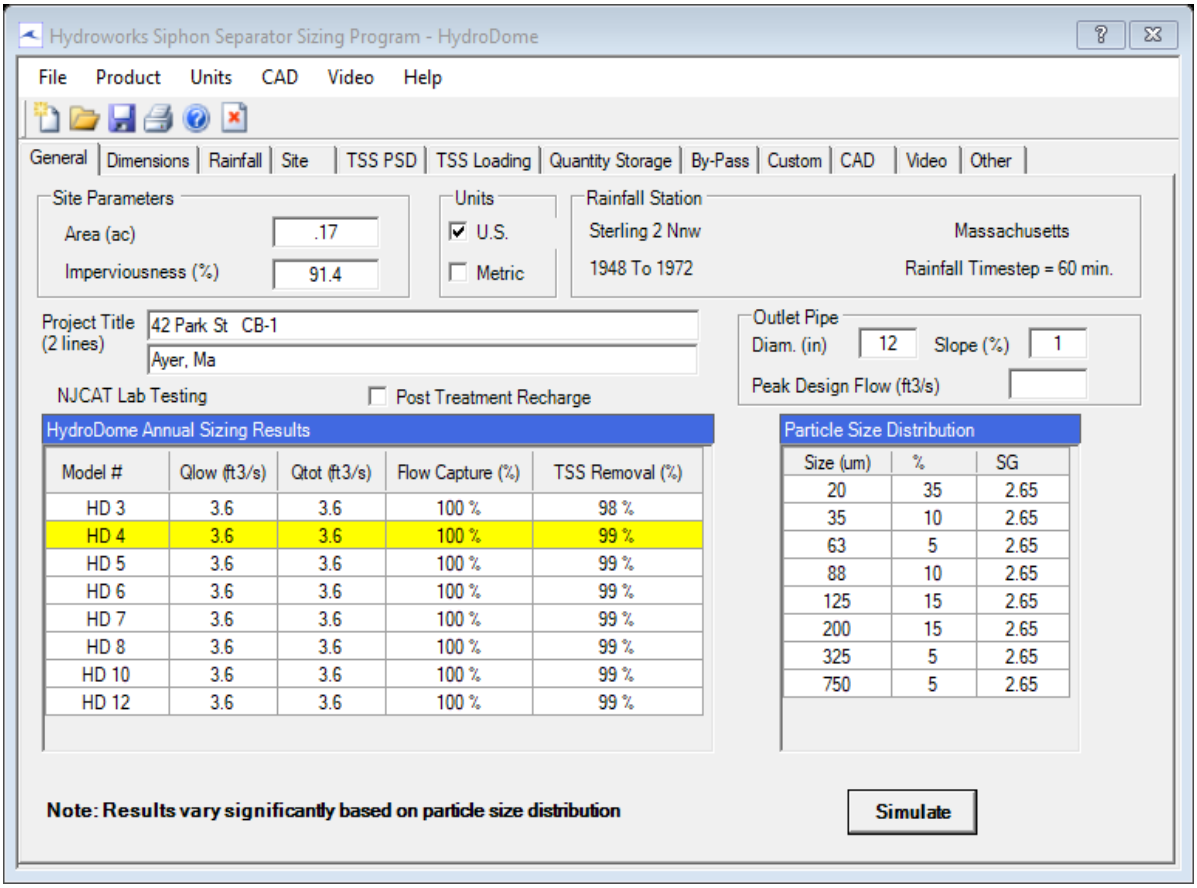
The HydroDome has a siphon which creates a discontinuity in headloss. Since a peak flow was not specified, headloss was calculated using the full pipe flow of 3.56 (ft3/s) for the given 12 (in) pipe diameter at 1% slope. The headloss was calculated to be 12 (in) above the crown of the 12 (in) outlet pipe.

This summary report provides the main parameters that were used for sizing. These parameters are shown on the summary tables and graphs provided in this report.

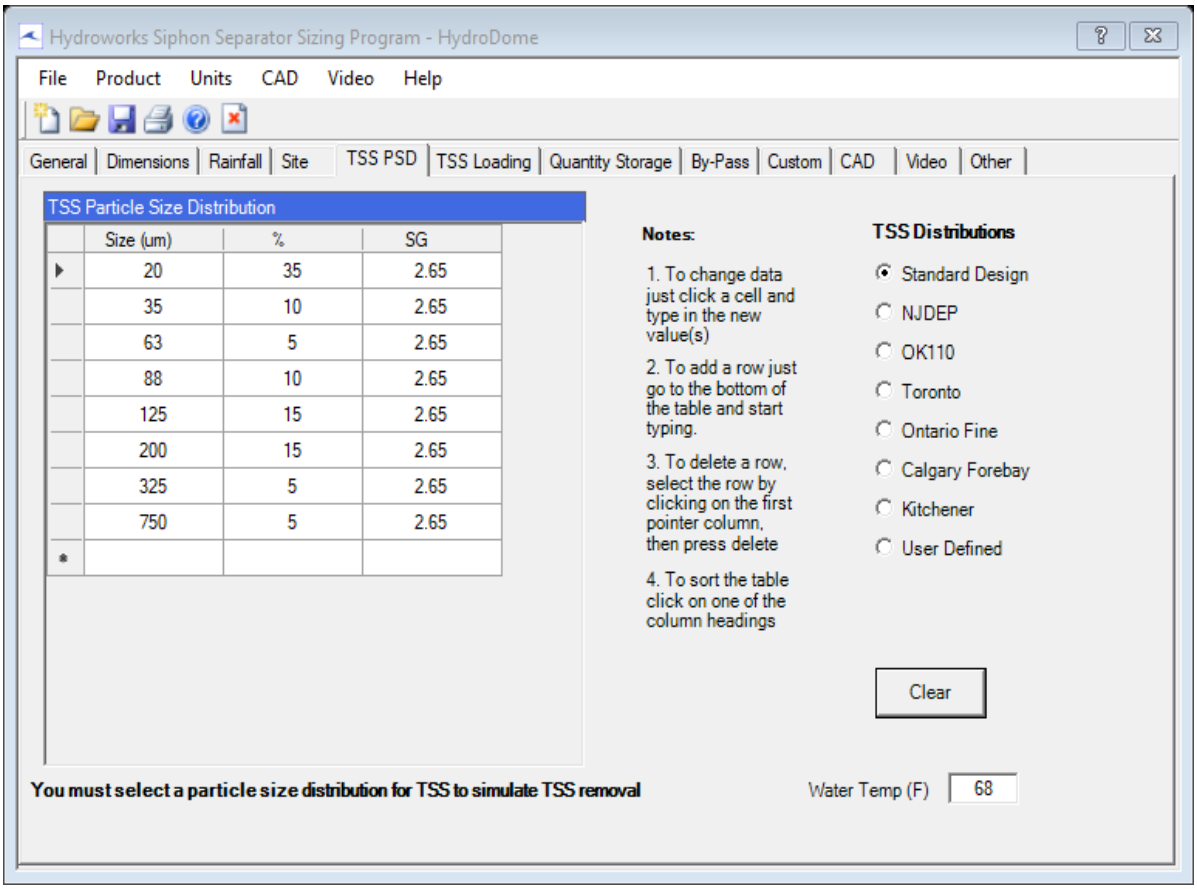
If you have any questions regarding this sizing summary please do not hesitate to contact Hydroworks at 888-290-7900 or email us at support@hydroworks.com.

The sizing program is for sizing purposes only and does not address any site specific parameters such as hydraulic gradeline, tailwater submergence, groundwater, soils bearing capacity, etc. Headloss calculations are not a hydraulic gradeline calculation since this requires a starting water level and an analysis of the entire system downstream of the HydroDome .

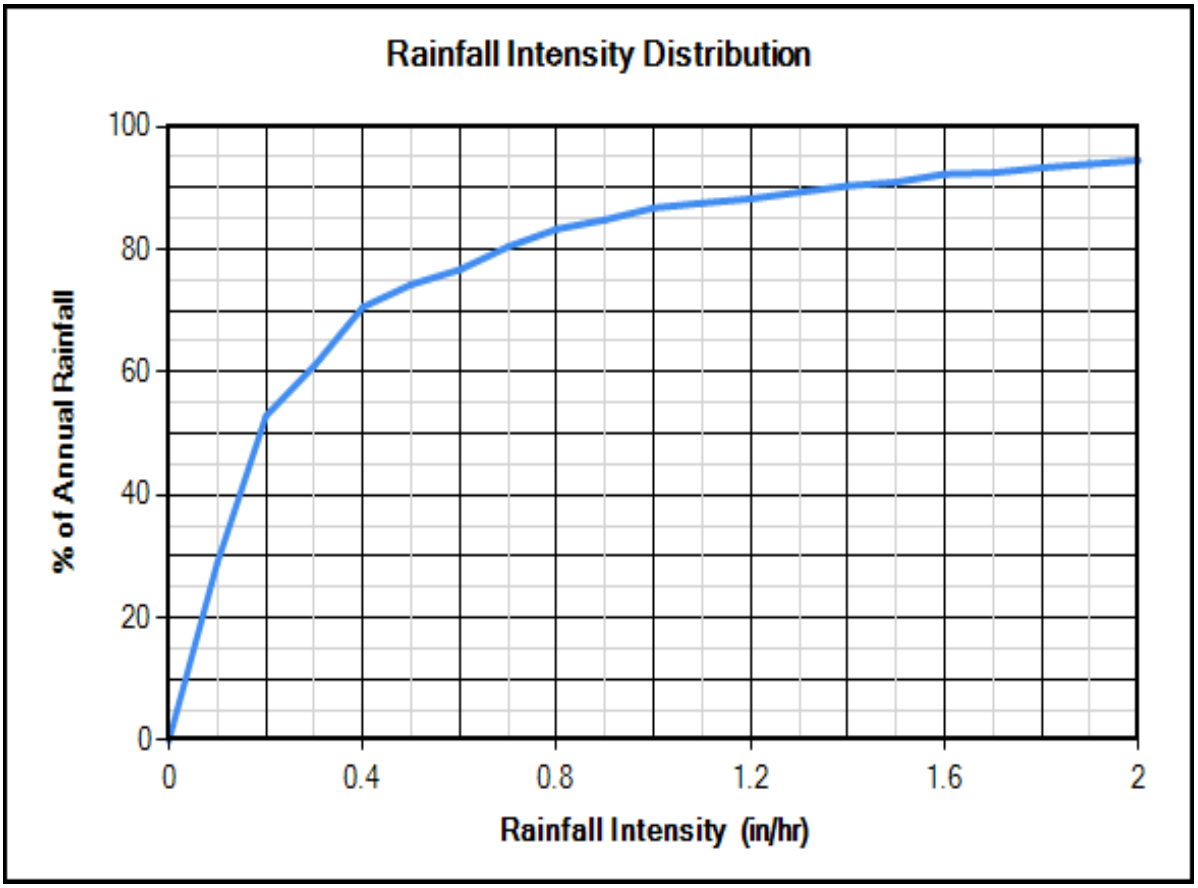
TSS Removal Sizing Summary



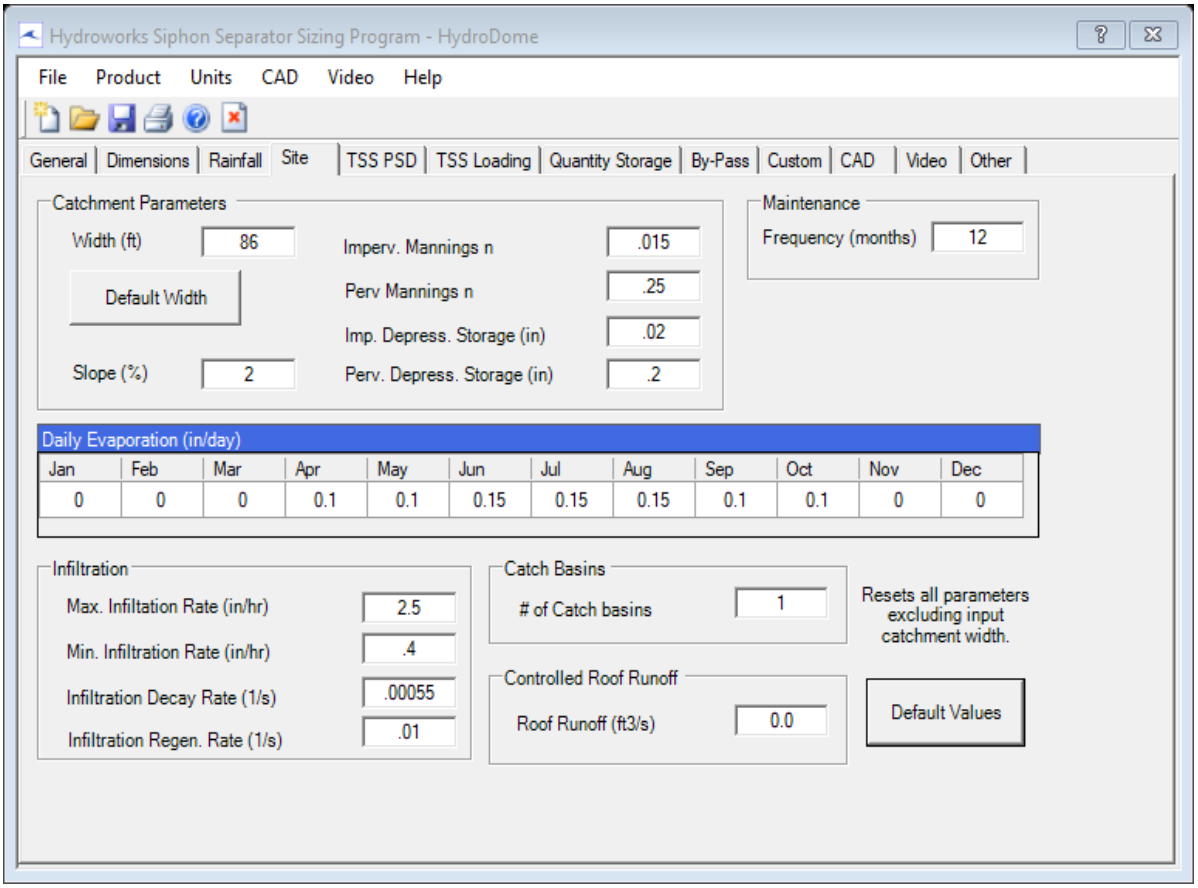
TSS Particle Size Distribution



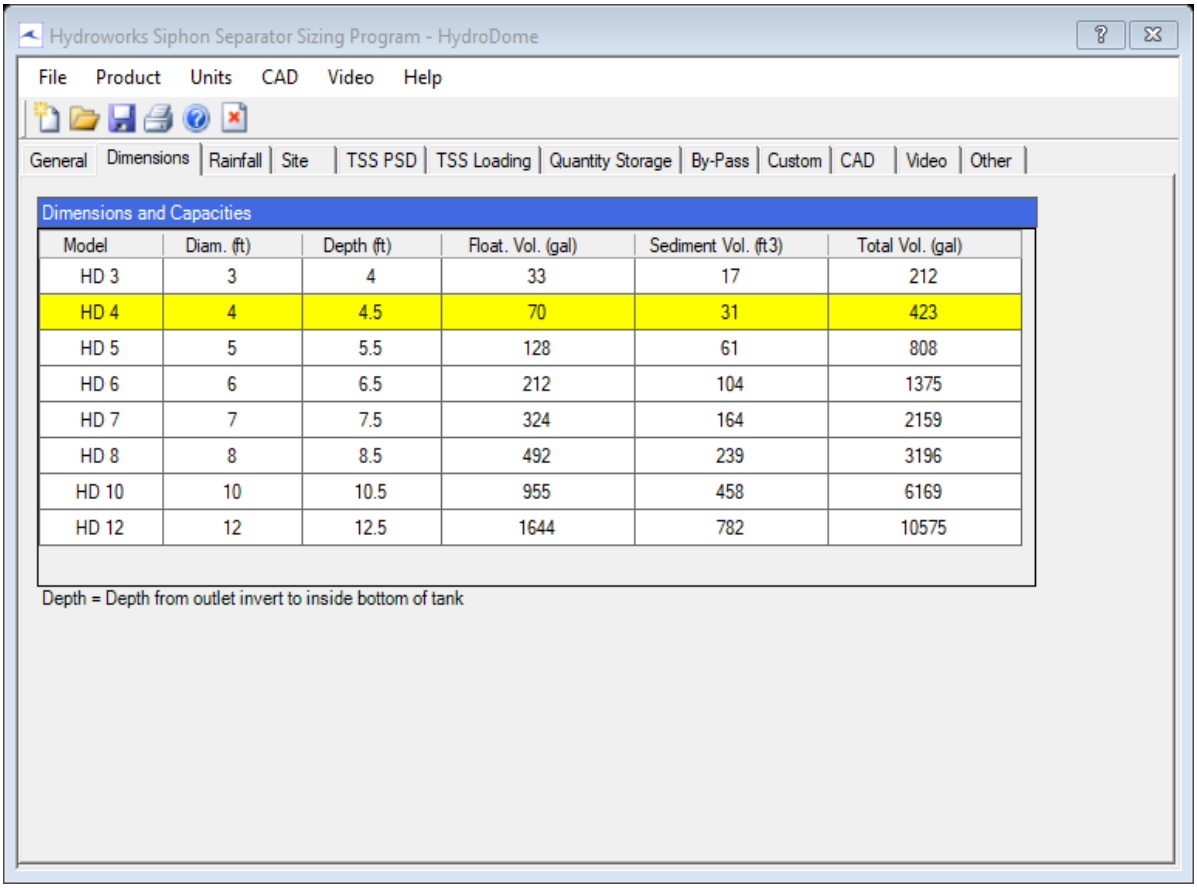
Rainfall Station - Sterling 2 Nnw, Massachusetts(1948 To 1972)



Site Physical Characteristics



Dimensions And Capacities

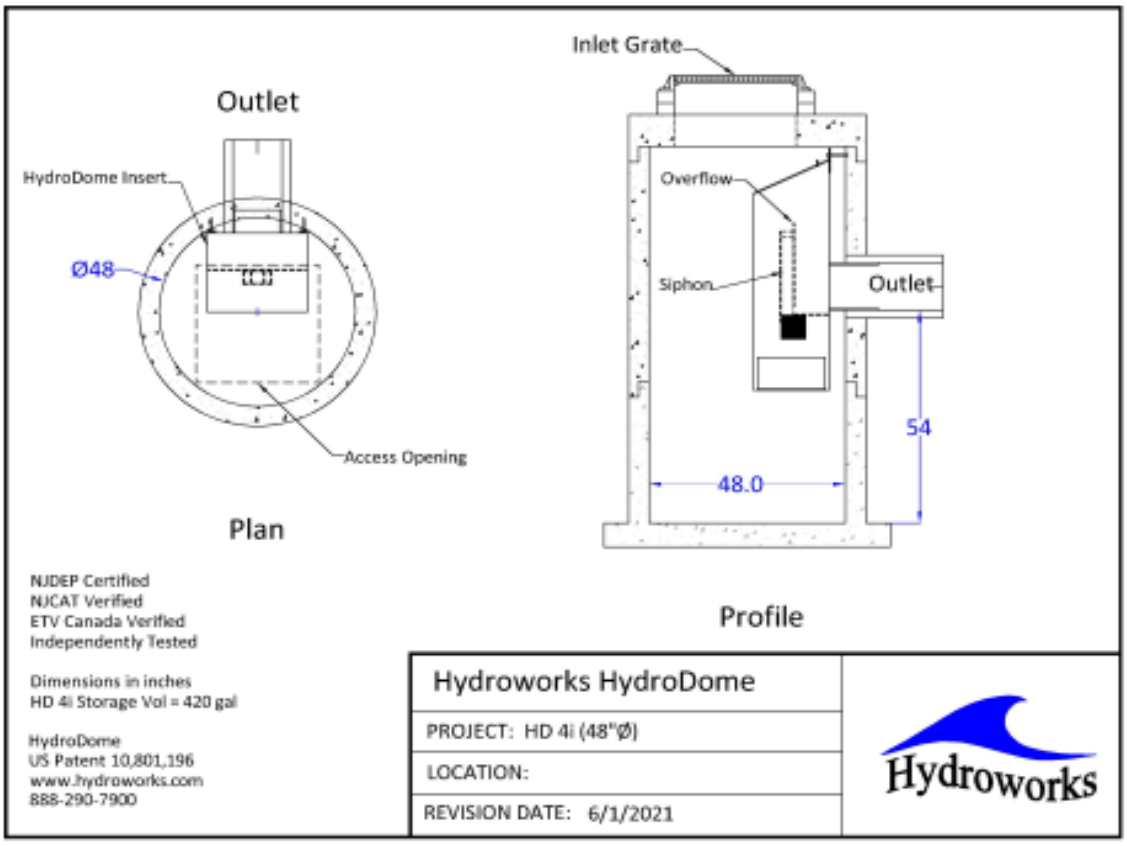


Dimensions and Capacities

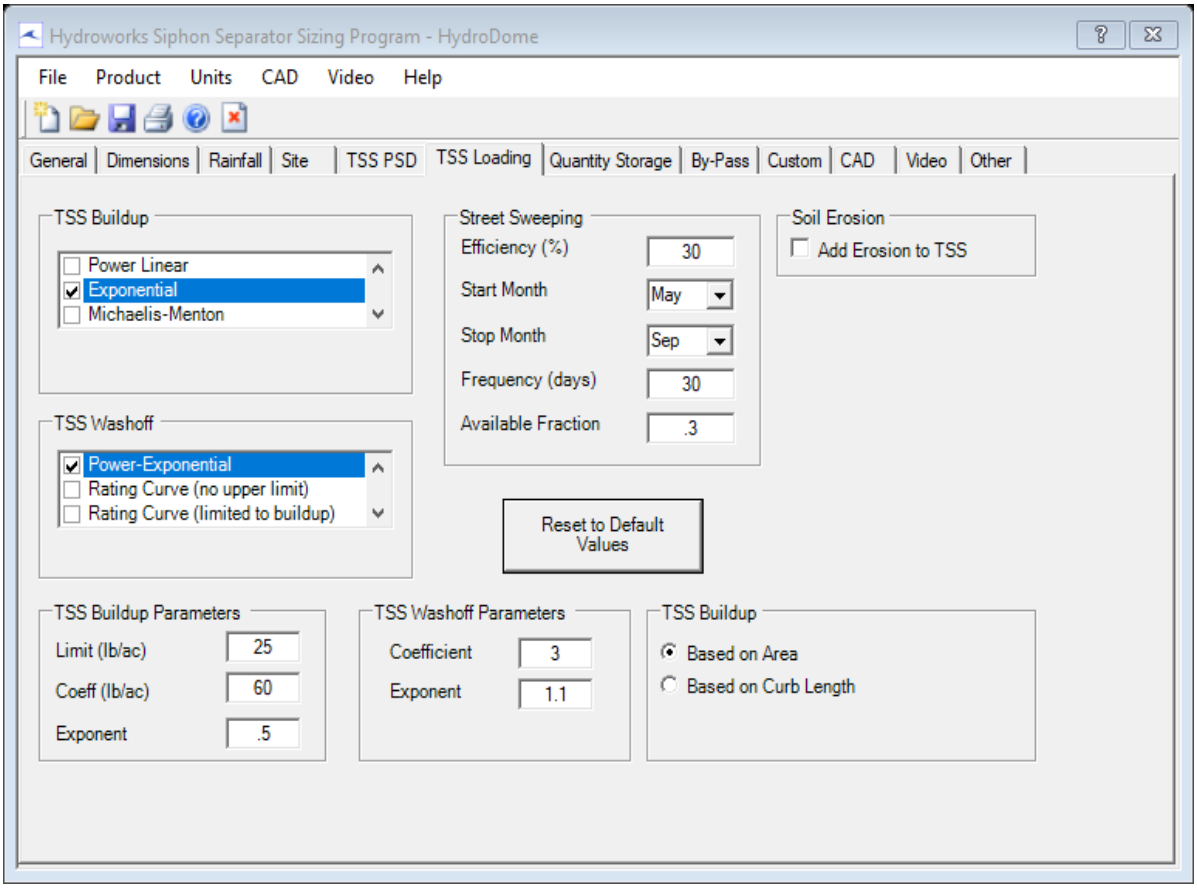
Model	Diam. (ft)	Depth (ft)	Float. Vol. (gal)	Sediment Vol. (ft3)	Total Vol. (gal)
HD 3	3	4	33	17	212
HD 4	4	4.5	70	31	423
HD 5	5	5.5	128	61	808
HD 6	6	6.5	212	104	1375
HD 7	7	7.5	324	164	2159
HD 8	8	8.5	492	239	3196
HD 10	10	10.5	955	458	6169
HD 12	12	12.5	1644	782	10575

Depth = Depth from outlet invert to inside bottom of tank

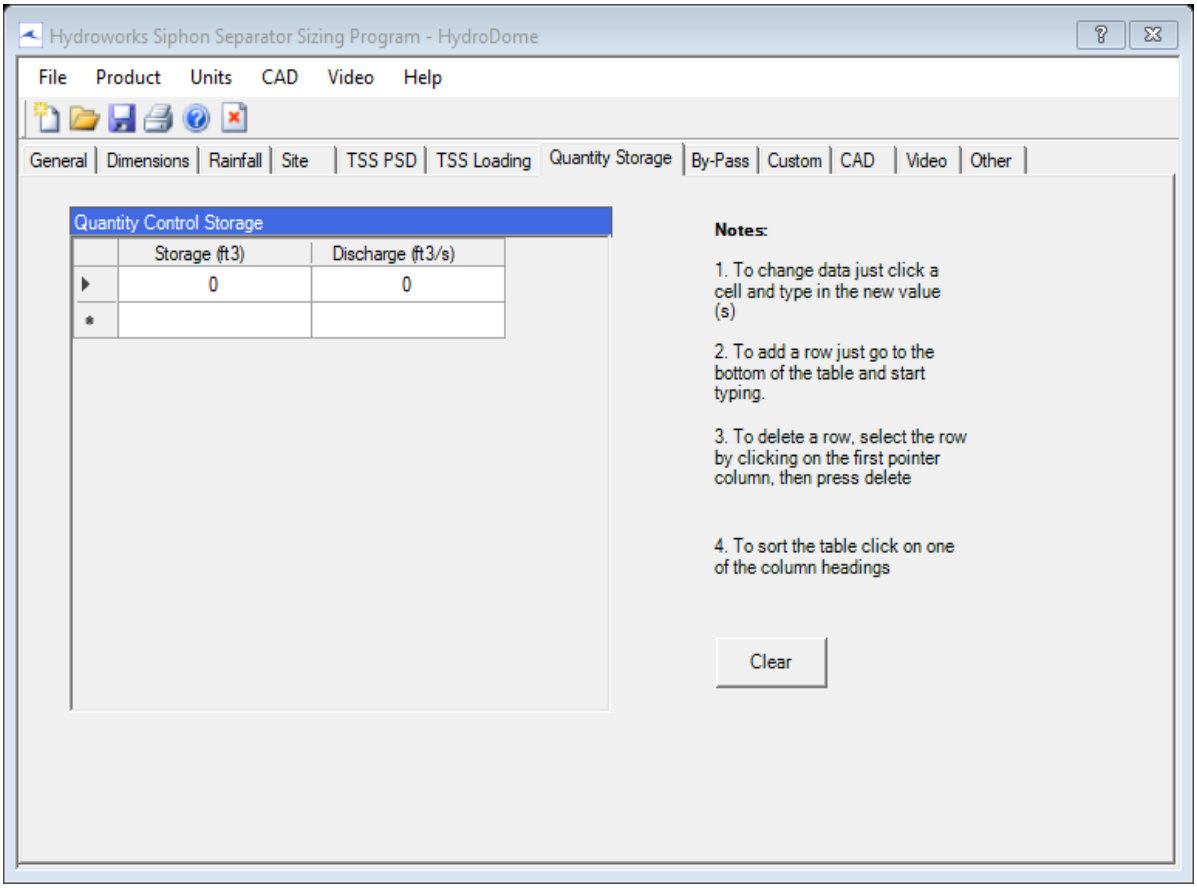
Generic HD 4i CAD Drawing



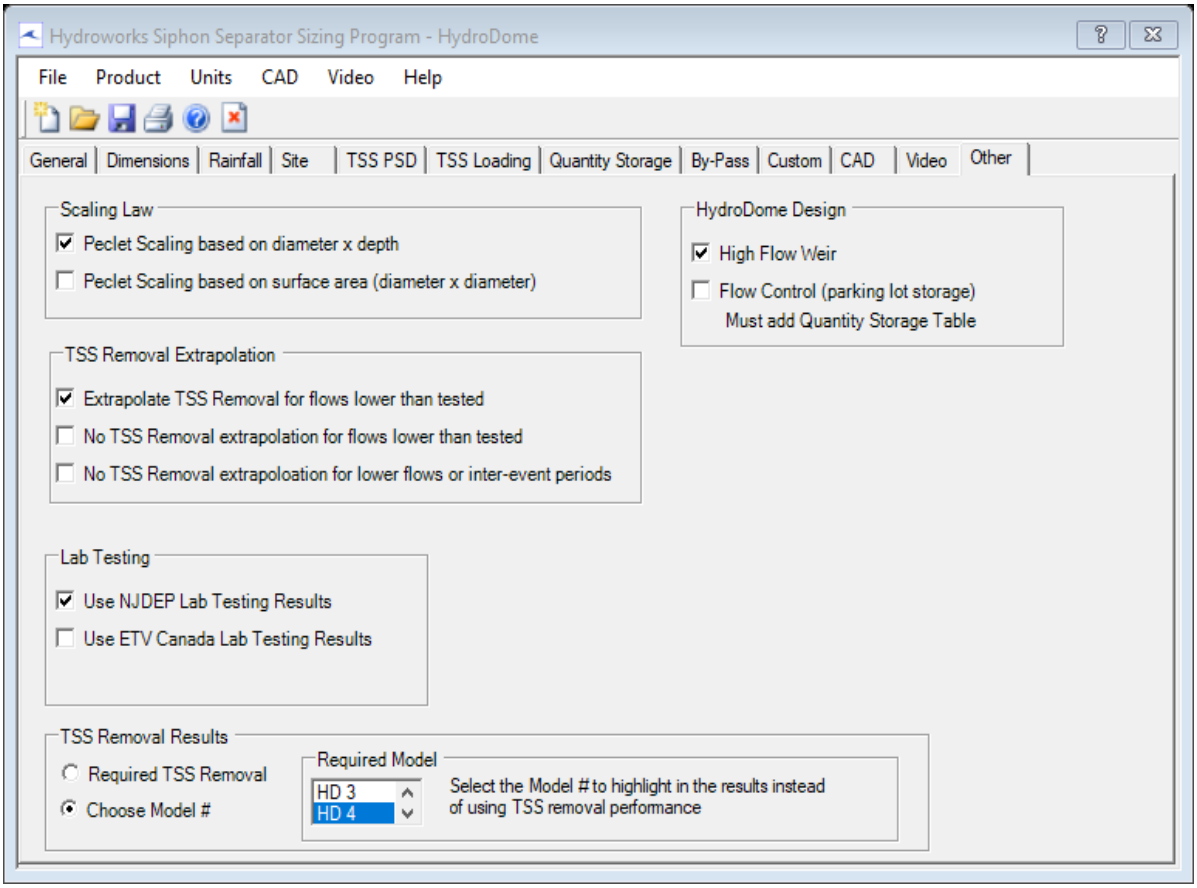
TSS Buildup And Washoff



Upstream Quantity Storage



Other Parameters



Flagged Issues

If there is underground detention storage upstream of the HydroDome please contact Hydroworks to ensure it has been modeled correctly.

Hydroworks Sizing Program - Version 5.6

1-800-290-7900
www.hydroworks.com

Hydroworks Sizing Summary

42 Park St CB-2

Ayer, Ma

08-21-2023

Recommended Size: HydroDome HD 4i

A HydroDome HD 4i is recommended to provide 99 % annual TSS removal based on a drainage area of .12 (ac) with an imperviousness of 39 % and Sterling 2 Nnw, Massachusetts rainfall for the Hydroworks standard particle size distribution.

The recommended HydroDome HD 4i treats 100 % of the annual runoff and provides 99 % annual TSS removal for the Sterling 2 Nnw rainfall records and Hydroworks standard particle size distribution.

The HydroDome has a siphon which creates a discontinuity in headloss. Since a peak flow was not specified, headloss was calculated using the full pipe flow of 3.56 (ft3/s) for the given 12 (in) pipe diameter at 1% slope. The headloss was calculated to be 12 (in) above the crown of the 12 (in) outlet pipe.

This summary report provides the main parameters that were used for sizing. These parameters are shown on the summary tables and graphs provided in this report.

If you have any questions regarding this sizing summary please do not hesitate to contact Hydroworks at 888-290-7900 or email us at support@hydroworks.com.

The sizing program is for sizing purposes only and does not address any site specific parameters such as hydraulic gradeline, tailwater submergence, groundwater, soils bearing capacity, etc. Headloss calculations are not a hydraulic gradeline calculation since this requires a starting water level and an analysis of the entire system downstream of the HydroDome .

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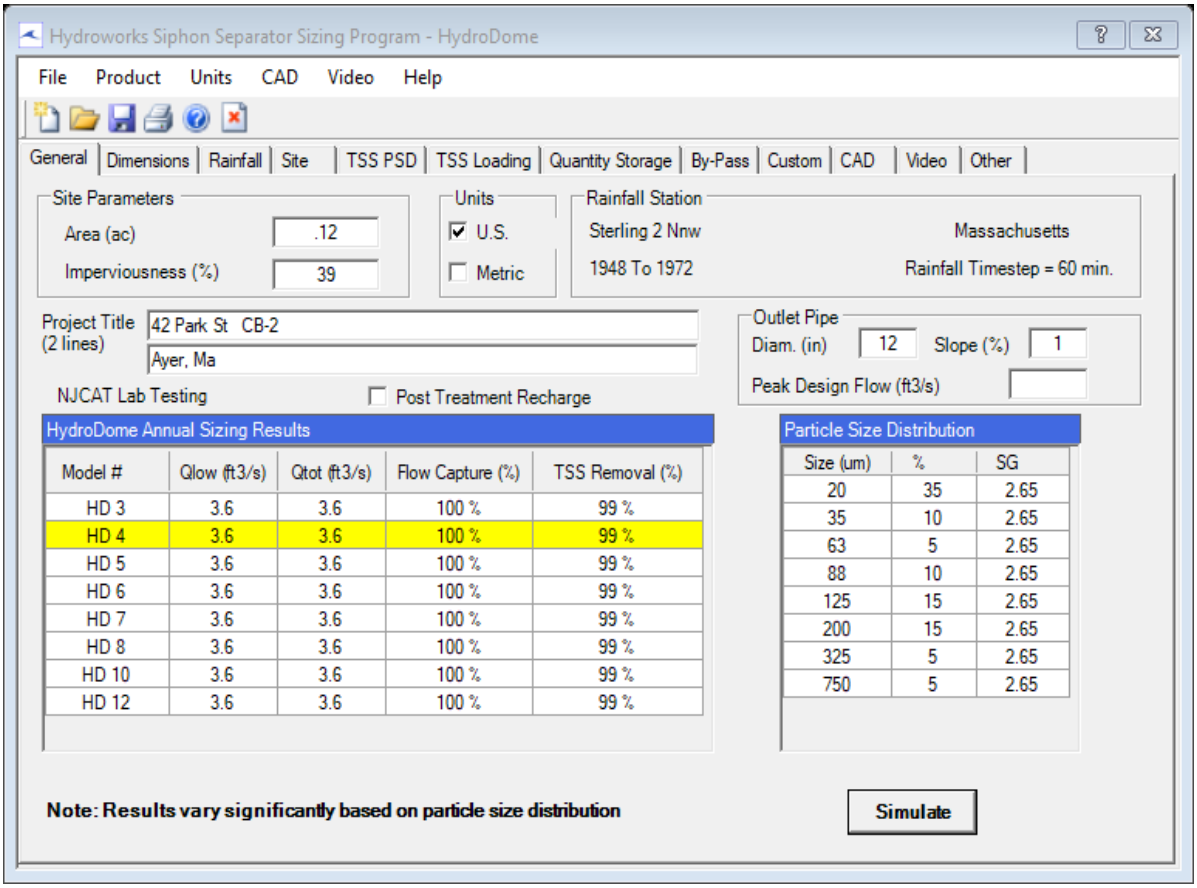

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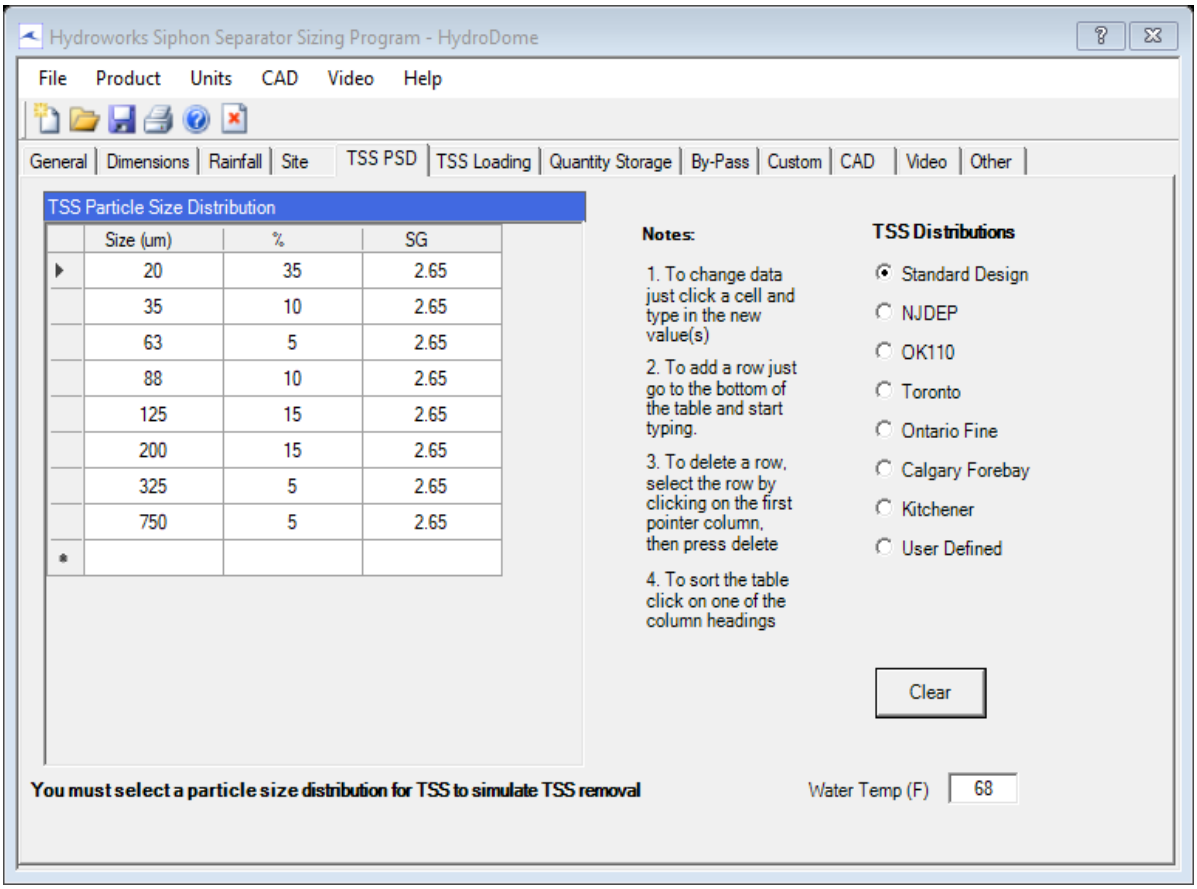

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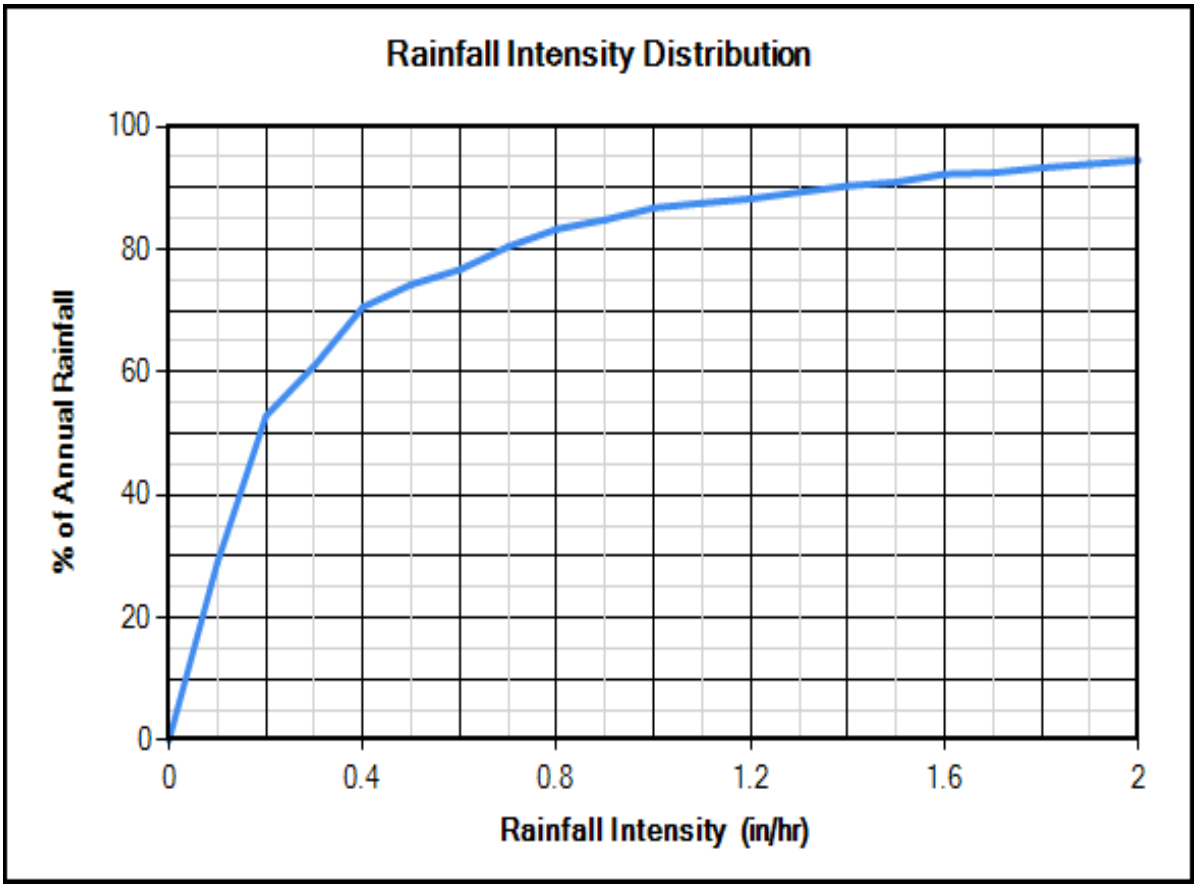

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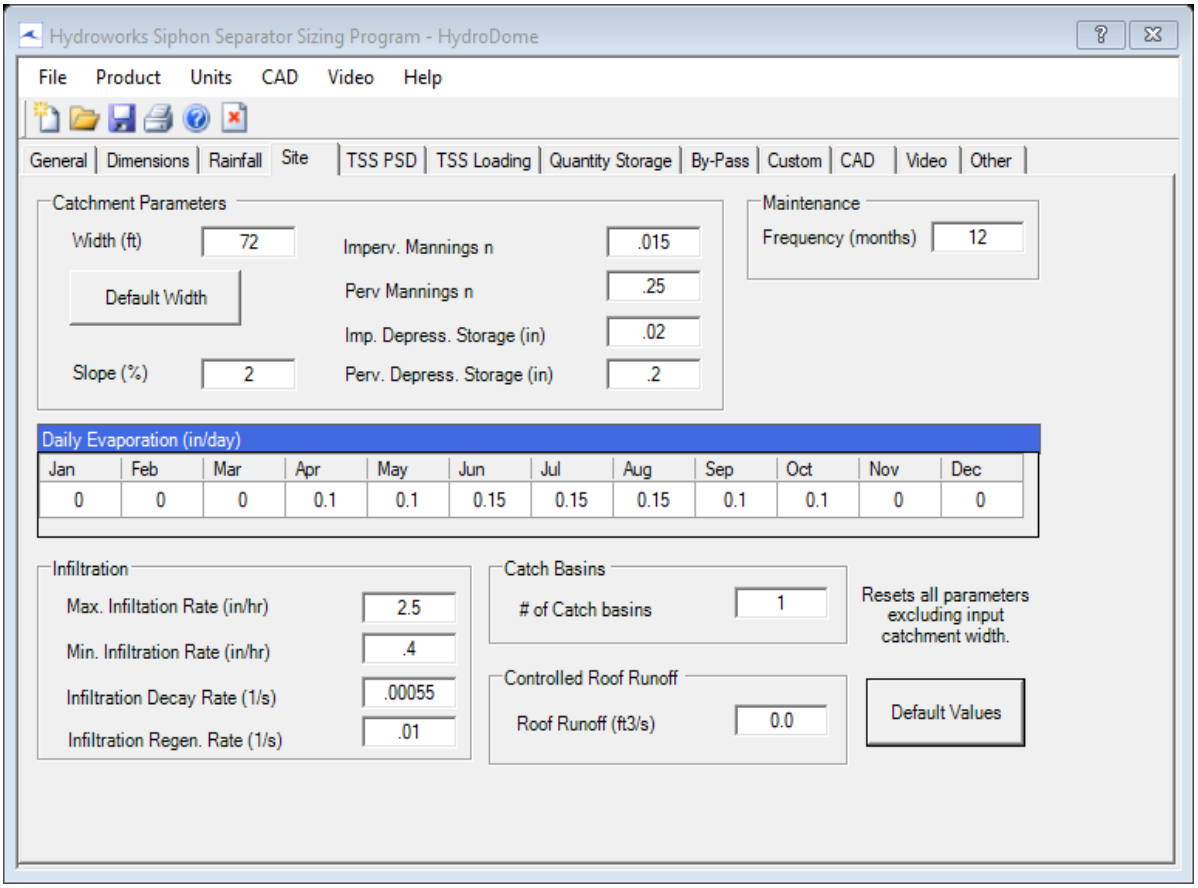
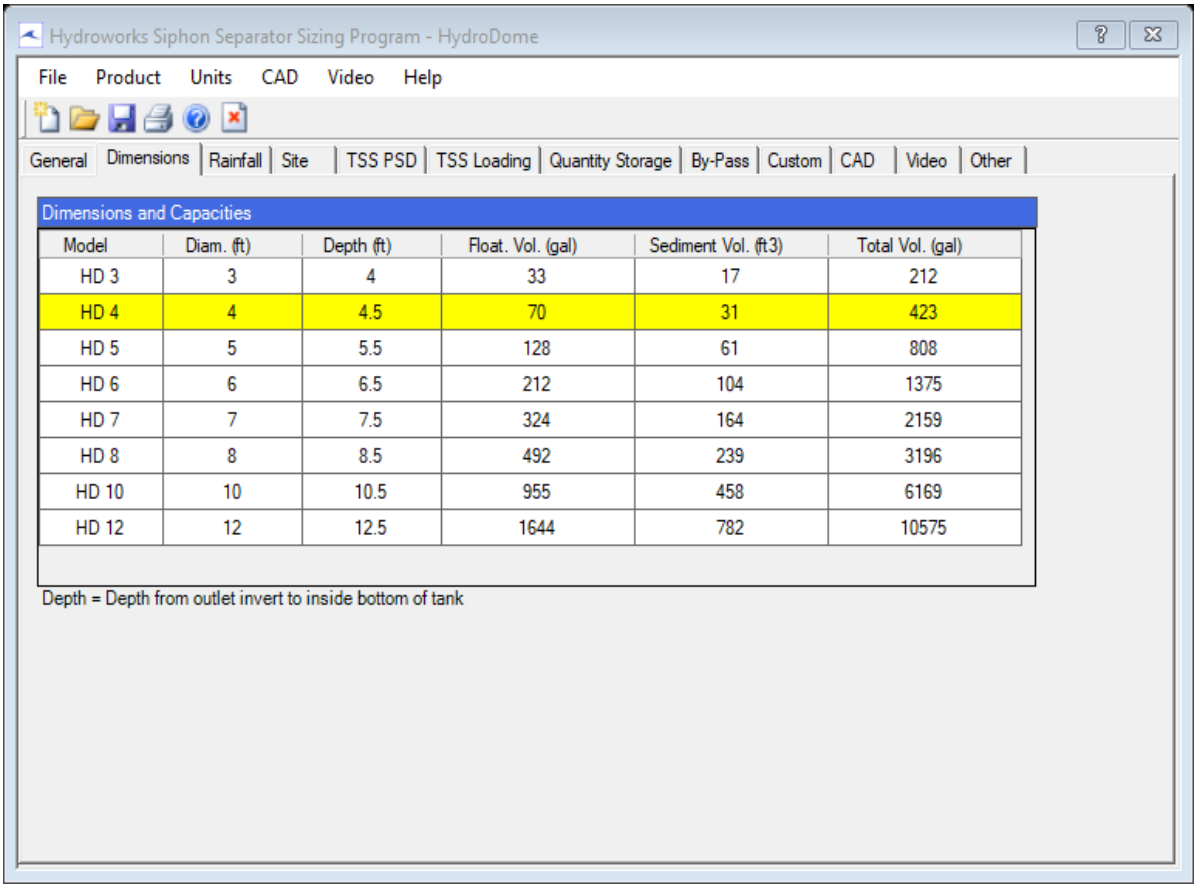


Dimensions And Capacities



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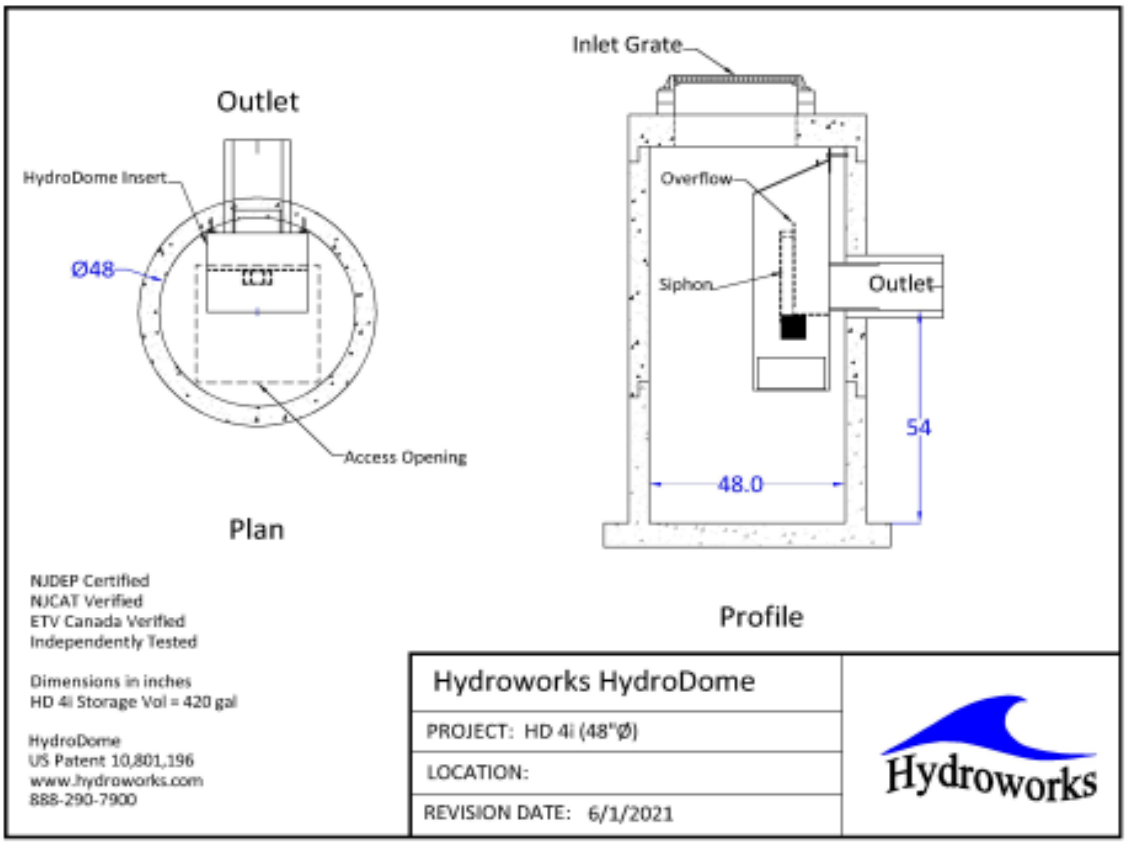
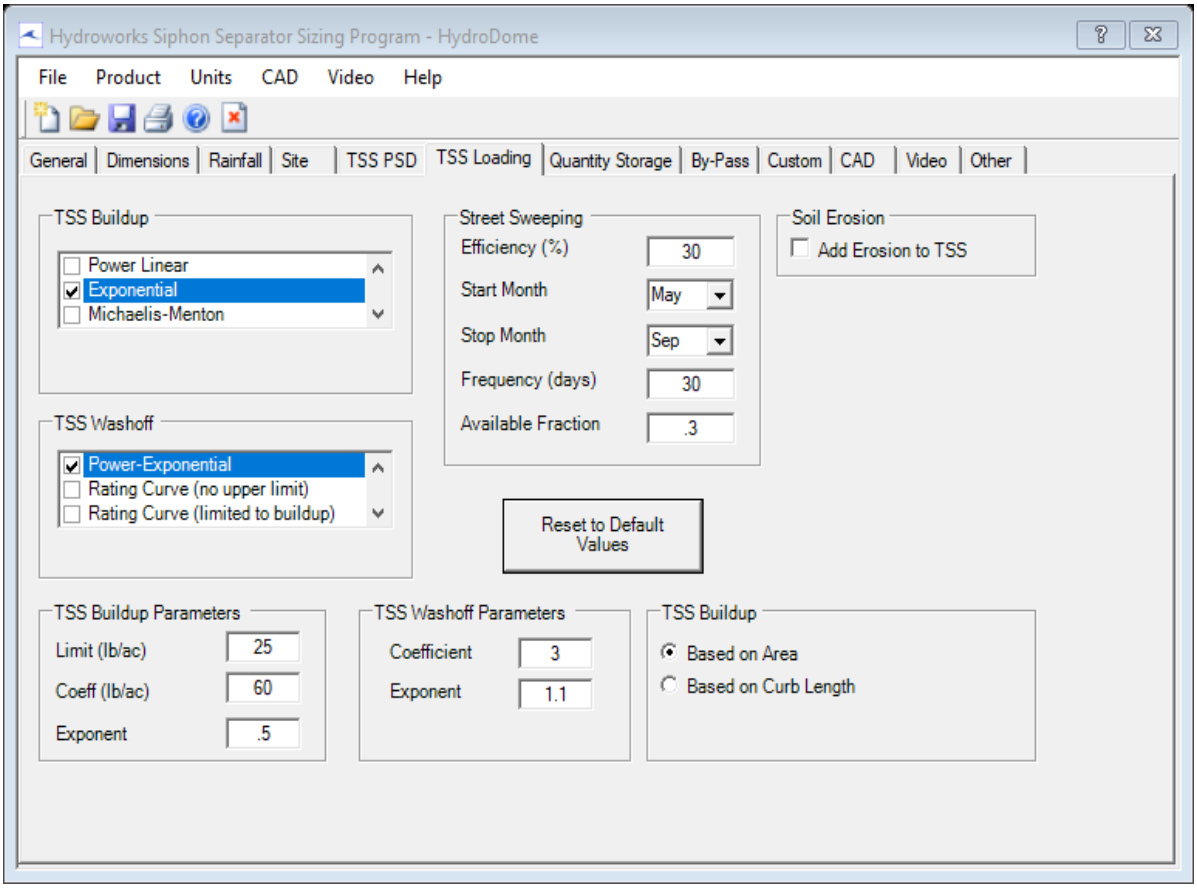


TSS Buildup And Washoff



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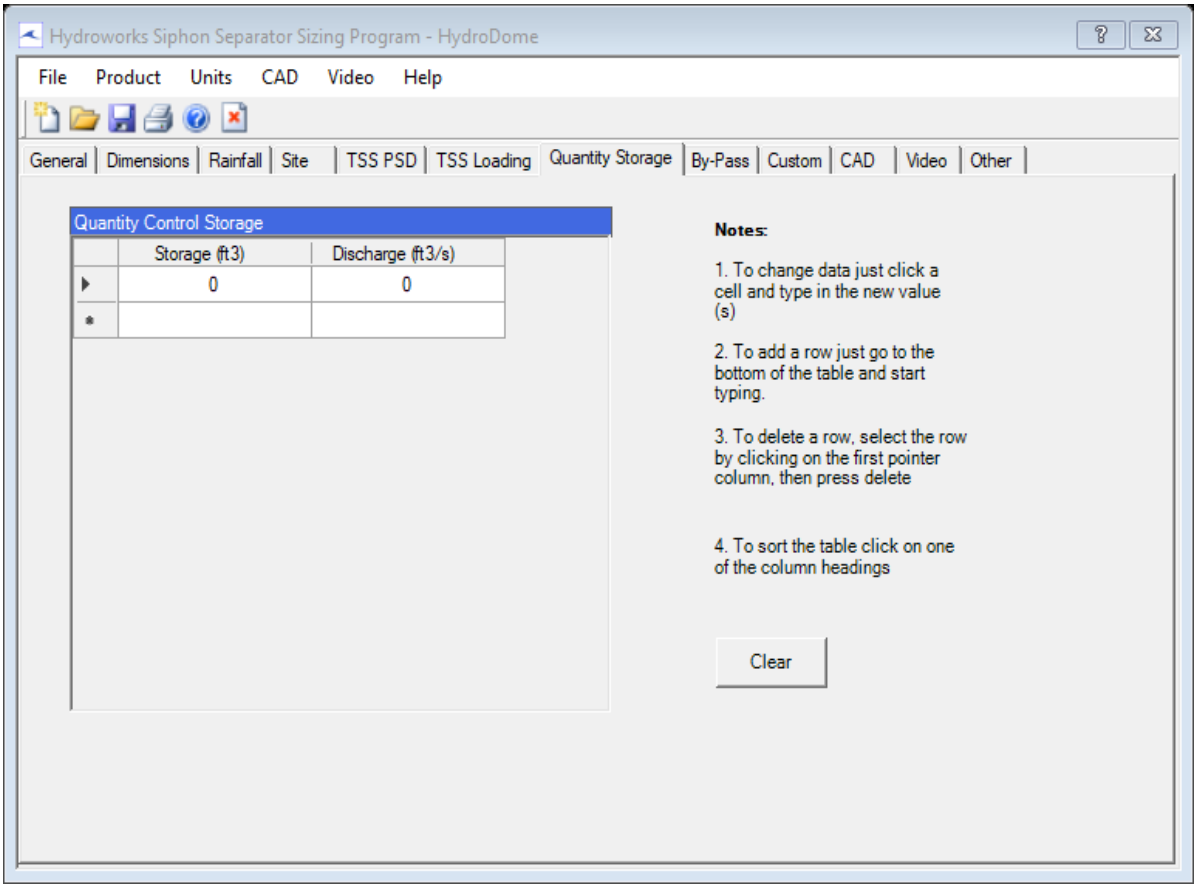
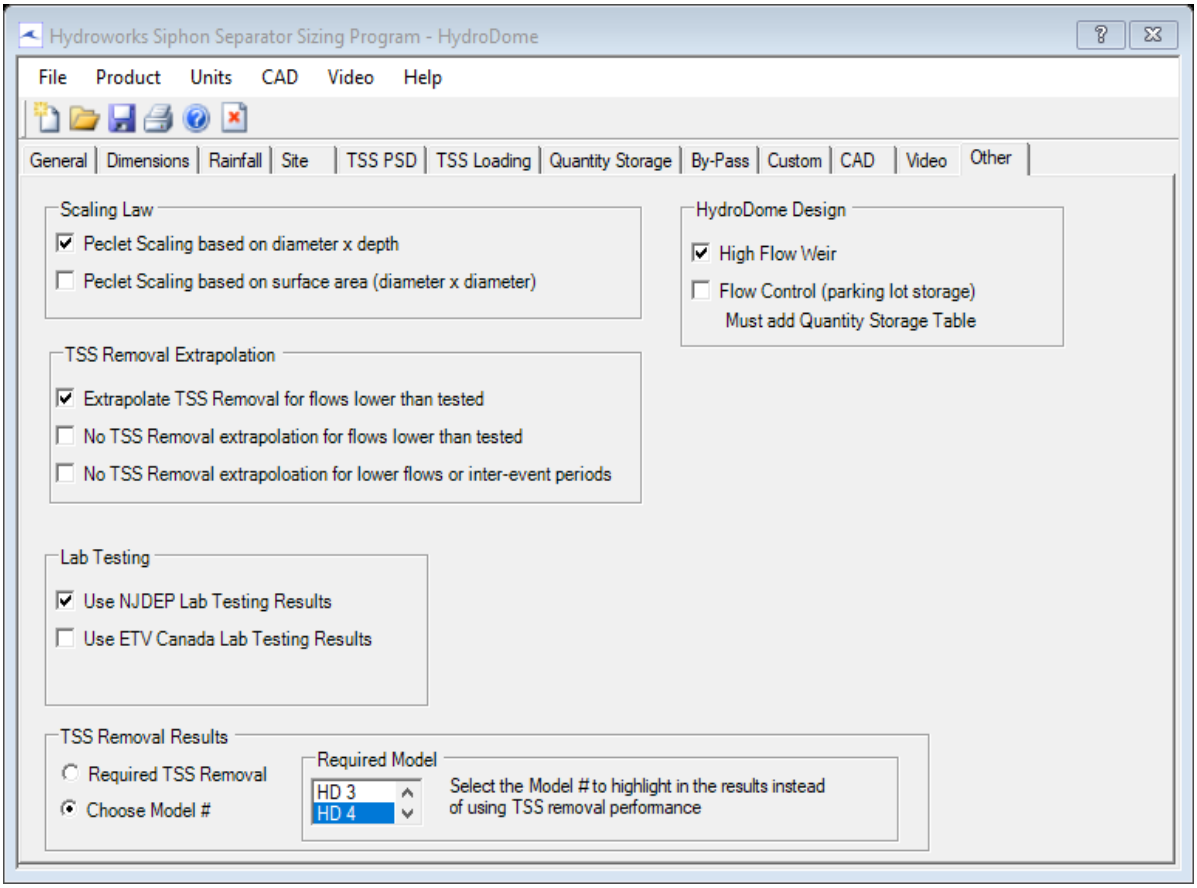


Other Parameters



Flagged Issues

If there is underground detention storage upstream of the HydroDome please contact Hydroworks to ensure it has been modeled correctly.

Hydroworks Sizing Program - Version 5.6

1-800-290-7900
www.hydroworks.com

Operation and Maintenance Plan

42 Park Street
Ayer, Massachusetts
Operation and Maintenance Plan

The site contractor will be responsible for the operation and maintenance of the stormwater collection system including deep sump catch basins and at grade infiltration basin during construction. After construction, the Property Owner is responsible for the operation and maintenance of the proposed stormwater collection system. The following Operation and Maintenance Plan for the project is proposed in accordance with DEP Stormwater Management Standard No. 9 to ensure that the stormwater collection and treatment system operates in accordance with the MADEP Stormwater Management Policy.

Schedule for Inspection and Maintenance after Construction:

Stormwater Management System Owner/Operator

- The property owner will be the owner and operator of the proposed stormwater collection system on site.
- If the property is sold, a copy of this Operation and Maintenance Plan will be transferred to the new property owners.

Below Grade Infiltration Basin

- Once constructed, basins will be inspected at a minimum after several storm events for the first year and annually thereafter to confirm drainage system functions as designed. Problems will be addressed immediately.
- System shall be cleaned as required per the manufacturer's recommendations.

Water Quality Units

- Structure cover should be inspected monthly for evidence of repair. Verify that inverts are secure and free flowing. Measure depth of sediment below water line.
- Unit shall be cleaned a minimum of twice per year. One of these cleanings to occur before April 15th of each year and one shall occur before September 15th of each year. Unite must be cleaned with a vacuum pump.

- All liquid, sediment, and hydrocarbons shall be pumped from the sump at least twice per year at intervals corresponding with the unit cleaning.
- All sediment, water and hydrocarbons should be properly handled and disposed of in accordance with local, state and federal guidelines and regulations.
- Refer to water quality unit manufacturers specifications for additional maintenance recommendations.

The routine and non-routine maintenance tasks to be undertaken after construction and a schedule for implementing those tasks.

- A site maintenance log will be kept. This log will record the dates when maintenance tasks were completed, the person who completed the task, and any observations of malfunctions in components of the stormwater management system. A sample maintenance log form is attached.

Estimated Operations and Maintenance Budget

- Operation and maintenance costs for the project are expected to be approximately $5,000/year

42 Park Street
Ayer, Massachusetts
Operation and Maintenance Plan

Operation and Maintenance Schedule

BMP	Frequency	Date Performed	Comments	Cleaning/ Repair Needed? Yes/No	Date of Cleaning/ Repair	Performed By
Below Grade Infiltration Basins	Inspection after each major storm event for the first year Cleaning as needed					
Water Quality Units	Monthly Inspections Biannual Cleaning					

Site Maintenance Supervisor:_____ Date:_____

Long Term Pollution Prevention Plan

42 Park Street
Ayer, Massachusetts
Long Term Pollution Prevention Plan

A long term pollution prevention plan is an important element of the routine operation and maintenance of an industrial facility that is designed to reduce or eliminate the creation of pollutants at the source. In addition to the obvious environmental benefits of protecting the natural resources downstream of the facility, maintaining a long term pollution prevention plan will provide for a healthier and safer living and work environment. The following long term pollution prevention practices will be employed at the property.

- Good housekeeping practices:
  Maintaining a clean property will prevent or reduce the amount of pollutants in the stormwater runoff discharging from the site. This will be achieved through periodic parking lot sweeping, at the owners discretion, and through catch basin and infiltration basin cleaning as detailed within the sites Stormwater Operation and Maintenance Plan.

- Provisions for storing materials and waste products inside or under cover:
  Materials will be stored in their appropriate containers and shall be stored under cover or in a secure enclosure to reduce the risk of spills. Waste products will be placed in proper bins until emptied by a licensed solid waste management company.

- Vehicle washing controls:
  Vehicle washing may be conducted within the driveways of the residential units. All wash water will be collected in the proposed stormwater infrastructure on site.

- Requirements for routine inspections and maintenance of stormwater BMPs:
  Refer to the maintenance schedule provided in the Stormwater Operation and Maintenance Plan.

- Spill prevention and response plans:
  Materials shall be stored in their proper original container in a secure location. No mixing of materials shall occur unless recommended by the manufacturer. The manufacturer's recommendations for proper use and disposal should be strictly adhered to. In the case of a spill the manufacturer's method for cleanup shall be followed. The area shall be kept ventilated and personnel handling the cleanup shall wear proper protective clothing. Spills of toxic or hazardous material shall be reported to the appropriate State and/or local authority in accordance with local and/or State regulations.

- Provisions for maintenance of lawns, gardens, and other landscaped areas:
  Owner will maintain surrounding landscaped area as needed.

- Requirements for storage and use of fertilizers, herbicides, and pesticides:
  Fertilizers, herbicides and pesticides shall be stored in their appropriate containers in a secure location as described above. Protective clothing shall be used when handled, and quantities shall be applied according to manufacturer's recommendations. Typically, the handling of these items will be the responsibility of a landscape contractor and will be stored off site.

- Pet waste management provisions:
  Pet waste management will be the responsibility of the individual pet owners. Trash receptacles will be located at various locations throughout the site for pet waste disposal.

- Provisions for operation and management of septic systems:
  Septic Systems are not applicable at this site.

- Provisions for solid waste management:
  Solid waste material shall be placed in outdoor secure containers until emptied by licensed waste management company.

- Snow disposal and plowing plans relative to Wetland Resource Areas:
  Snow shall be placed on upland areas only where sand and debris will remain after snowmelt for later removal. Snow shall be plowed in accordance with standard operating procedures and stored in designated areas as detailed on the site plan approval documents. Any sand and debris remaining after snow piles have melted will be removed by the facility owners or maintenance contractors.

- Winter Road Salt/or Sand Use and Storage restriction:
  The use of environmentally friendly alternatives to road salt will be considered.

- Street sweeping schedules
  Street sweeping will occur only as needed at the discretion of the owner.

- Provisions for prevention of illicit discharges to the stormwater management system:
  The Stormwater Management System associated with the development has been designed such that prior to storm water runoff discharging from the site, it is treated through a series of best management practices. To the Engineer's knowledge, there are no known or designed non-storm water discharges that are or will be connected to the storm water collection system that would convey pollutants directly to groundwater or surface waters.

- Documentation that Stormwater BMPs are designed to provide for shutdown and containment in the event of a spill or discharges to a near critical areas or from LUHPPL:
  All catch basins shall be equipped with hoods to prevent oils and floatables from discharging to the underground infiltration basins.

- Training for staff or personnel involved with the implementing Long Term Pollution Prevention Plan:
  Facilities staff will be responsible for implementing the Long Term Pollution Prevention Plan and staff will be trained in accordance with company policy.

- List of Emergency contacts for implementing Long-Term Pollution Prevention Plan:
  Ali Goldinak
  50 Mountain Ave,
  Fitchburg, AM 01420
  (973) 202-6333

MADEP Stormwater Checklist

Massachusetts Department of Environmental Protection
Bureau of Resource Protection - Wetlands Program

Checklist for Stormwater Report

B. Stormwater Checklist and Certification

The following checklist is intended to serve as a guide for applicants as to the elements that ordinarily need to be addressed in a complete Stormwater Report. The checklist is also intended to provide conservation commissions and other reviewing authorities with a summary of the components necessary for a comprehensive Stormwater Report that addresses the ten Stormwater Standards.

Note: Because stormwater requirements vary from project to project, it is possible that a complete Stormwater Report may not include information on some of the subjects specified in the Checklist. If it is determined that a specific item does not apply to the project under review, please note that the item is not applicable (N.A.) and provide the reasons for that determination.

A complete checklist must include the Certification set forth below signed by the Registered Professional Engineer who prepared the Stormwater Report.

Registered Professional Engineer's Certification

I have reviewed the Stormwater Report, including the soil evaluation, computations, Long-term Pollution Prevention Plan, the Construction Period Erosion and Sedimentation Control Plan (if included), the Long-term Post-Construction Operation and Maintenance Plan, the Illicit Discharge Compliance Statement (if included) and the plans showing the stormwater management system, and have determined that they have been prepared in accordance with the requirements of the Stormwater Management Standards as further elaborated by the Massachusetts Stormwater Handbook. I have also determined that the information presented in the Stormwater Checklist is accurate and that the information presented in the Stormwater Report accurately reflects conditions at the site as of the date of this permit application.

Registered Professional Engineer Block and Signature

9/1/2023

Signature and Date

Checklist

Project Type: Is the application for new development, redevelopment, or a mix of new and redevelopment?

☒ New development

☐ Redevelopment

☐ Mix of New Development and Redevelopment

Checklist for Stormwater Report

Checklist (continued)

LID Measures: Stormwater Standards require LID measures to be considered. Document what environmentally sensitive design and LID Techniques were considered during the planning and design of the project:

- ☐ No disturbance to any Wetland Resource Areas
- ☐ Site Design Practices (e.g. clustered development, reduced frontage setbacks)
- ☐ Reduced Impervious Area (Redevelopment Only)
- ☐ Minimizing disturbance to existing trees and shrubs
- ☐ LID Site Design Credit Requested:
  - ☐ Credit 1
  - ☐ Credit 2
  - ☐ Credit 3
- ☐ Use of “country drainage” versus curb and gutter conveyance and pipe
- ☐ Bioretention Cells (includes Rain Gardens)
- ☐ Constructed Stormwater Wetlands (includes Gravel Wetlands designs)
- ☐ Treebox Filter
- ☐ Water Quality Swale
- ☐ Grass Channel
- ☐ Green Roof
- ☐ Other (describe): _____

Standard 1: No New Untreated Discharges

- ☒ No new untreated discharges
- ☐ Outlets have been designed so there is no erosion or scour to wetlands and waters of the Commonwealth
- ☒ Supporting calculations specified in Volume 3 of the Massachusetts Stormwater Handbook included.

Checklist for Stormwater Report

Checklist (continued)

Standard 2: Peak Rate Attenuation

☐ Standard 2 waiver requested because the project is located in land subject to coastal storm flowage and stormwater discharge is to a wetland subject to coastal flooding.

☐ Evaluation provided to determine whether off-site flooding increases during the 100-year 24-hour storm.

☒ Calculations provided to show that post-development peak discharge rates do not exceed pre-development rates for the 2-year and 10-year 24-hour storms. If evaluation shows that off-site flooding increases during the 100-year 24-hour storm, calculations are also provided to show that post-development peak discharge rates do not exceed pre-development rates for the 100-year 24-hour storm.

Standard 3: Recharge

☐ Soil Analysis provided.

☒ Required Recharge Volume calculation provided.

☐ Required Recharge volume reduced through use of the LID site Design Credits.

☒ Sizing the infiltration, BMPs is based on the following method: Check the method used.

☒ Static ☐ Simple Dynamic ☐ Dynamic Field[1]

☒ Runoff from all impervious areas at the site discharging to the infiltration BMP.

☐ Runoff from all impervious areas at the site is *not* discharging to the infiltration BMP and calculations are provided showing that the drainage area contributing runoff to the infiltration BMPs is sufficient to generate the required recharge volume.

☒ Recharge BMPs have been sized to infiltrate the Required Recharge Volume.

☐ Recharge BMPs have been sized to infiltrate the Required Recharge Volume *only* to the maximum extent practicable for the following reason:

- ☐ Site is comprised solely of C and D soils and/or bedrock at the land surface
- ☐ M.G.L. c. 21E sites pursuant to 310 CMR 40.0000
- ☐ Solid Waste Landfill pursuant to 310 CMR 19.000
- ☐ Project is otherwise subject to Stormwater Management Standards only to the maximum extent practicable.

☒ Calculations showing that the infiltration BMPs will drain in 72 hours are provided.

☐ Property includes a M.G.L. c. 21E site or a solid waste landfill and a mounding analysis is included.

[1] 80% TSS removal is required prior to discharge to infiltration BMP if Dynamic Field method is used.

Checklist for Stormwater Report

Checklist (continued)

Standard 3: Recharge (continued)

☐ The infiltration BMP is used to attenuate peak flows during storms greater than or equal to the 10-year 24-hour storm and separation to seasonal high groundwater is less than 4 feet and a mounding analysis is provided.

☐ Documentation is provided showing that infiltration BMPs do not adversely impact nearby wetland resource areas.

Standard 4: Water Quality

The Long-Term Pollution Prevention Plan typically includes the following:

- Good housekeeping practices;
- Provisions for storing materials and waste products inside or under cover;
- Vehicle washing controls;
- Requirements for routine inspections and maintenance of stormwater BMPs;
- Spill prevention and response plans;
- Provisions for maintenance of lawns, gardens, and other landscaped areas;
- Requirements for storage and use of fertilizers, herbicides, and pesticides;
- Pet waste management provisions;
- Provisions for operation and management of septic systems;
- Provisions for solid waste management;
- Snow disposal and plowing plans relative to Wetland Resource Areas;
- Winter Road Salt and/or Sand Use and Storage restrictions;
- Street sweeping schedules;
- Provisions for prevention of illicit discharges to the stormwater management system;
- Documentation that Stormwater BMPs are designed to provide for shutdown and containment in the event of a spill or discharges to or near critical areas or from LUHPPL;
- Training for staff or personnel involved with implementing Long-Term Pollution Prevention Plan;
- List of Emergency contacts for implementing Long-Term Pollution Prevention Plan.

☒ A Long-Term Pollution Prevention Plan is attached to Stormwater Report and is included as an attachment to the Wetlands Notice of Intent.

☒ Treatment BMPs subject to the 44% TSS removal pretreatment requirement and the one inch rule for calculating the water quality volume are included, and discharge:

- ☐ is within the Zone II or Interim Wellhead Protection Area
- ☐ is near or to other critical areas
- ☒ is within soils with a rapid infiltration rate (greater than 2.4 inches per hour)
- ☐ involves runoff from land uses with higher potential pollutant loads.

☐ The Required Water Quality Volume is reduced through use of the LID site Design Credits.

☐ Calculations documenting that the treatment train meets the 80% TSS removal requirement and, if applicable, the 44% TSS removal pretreatment requirement, are provided.

Checklist for Stormwater Report

Checklist (continued)

Standard 4: Water Quality (continued)

☒ The BMP is sized (and calculations provided) based on:

- ☒ The ½" or 1" Water Quality Volume or
- ☐ The equivalent flow rate associated with the Water Quality Volume and documentation is provided showing that the BMP treats the required water quality volume.

☐ The applicant proposes to use proprietary BMPs, and documentation supporting use of proprietary BMP and proposed TSS removal rate is provided. This documentation may be in the form of the propriety BMP checklist found in Volume 2, Chapter 4 of the Massachusetts Stormwater Handbook and submitting copies of the TARP Report, STEP Report, and/or other third party studies verifying performance of the proprietary BMPs.

☐ A TMDL exists that indicates a need to reduce pollutants other than TSS and documentation showing that the BMPs selected are consistent with the TMDL is provided.

Standard 5: Land Uses With Higher Potential Pollutant Loads (LUHPPLs)

☐ The NPDES Multi-Sector General Permit covers the land use and the Stormwater Pollution Prevention Plan (SWPPP) has been included with the Stormwater Report.

☐ The NPDES Multi-Sector General Permit covers the land use and the SWPPP will be submitted ***prior to*** the discharge of stormwater to the post-construction stormwater BMPs.

☒ The NPDES Multi-Sector General Permit does ***not*** cover the land use.

☐ LUHPPLs are located at the site and industry specific source control and pollution prevention measures have been proposed to reduce or eliminate the exposure of LUHPPLs to rain, snow, snow melt and runoff, and been included in the long term Pollution Prevention Plan.

☐ All exposure has been eliminated.

☐ All exposure has ***not*** been eliminated and all BMPs selected are on MassDEP LUHPPL list.

☐ The LUHPPL has the potential to generate runoff with moderate to higher concentrations of oil and grease (e.g. all parking lots with >1000 vehicle trips per day) and the treatment train includes an oil grit separator, a filtering bioretention area, a sand filter or equivalent.

Standard 6: Critical Areas

☐ The discharge is near or to a critical area and the treatment train includes only BMPs that MassDEP has approved for stormwater discharges to or near that particular class of critical area.

☐ Critical areas and BMPs are identified in the Stormwater Report.

Checklist for Stormwater Report

Checklist (continued)

Standard 7: Redevelopments and Other Projects Subject to the Standards only to the maximum extent practicable

☐ The project is subject to the Stormwater Management Standards only to the maximum Extent Practicable as a:

- ☐ Limited Project
- ☐ Small Residential Projects: 5-9 single family houses or 5-9 units in a multi-family development provided there is no discharge that may potentially affect a critical area.
- ☐ Small Residential Projects: 2-4 single family houses or 2-4 units in a multi-family development with a discharge to a critical area
- ☐ Marina and/or boatyard provided the hull painting, service and maintenance areas are protected from exposure to rain, snow, snow melt and runoff
- ☐ Bike Path and/or Foot Path
- ☐ Redevelopment Project
- ☐ Redevelopment portion of mix of new and redevelopment.

☐ Certain standards are not fully met (Standard No. 1, 8, 9, and 10 must always be fully met) and an explanation of why these standards are not met is contained in the Stormwater Report.

☐ The project involves redevelopment and a description of all measures that have been taken to improve existing conditions is provided in the Stormwater Report. The redevelopment checklist found in Volume 2 Chapter 3 of the Massachusetts Stormwater Handbook may be used to document that the proposed stormwater management system (a) complies with Standards 2, 3 and the pretreatment and structural BMP requirements of Standards 4-6 to the maximum extent practicable and (b) improves existing conditions.

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan must include the following information:

- Narrative;
- Construction Period Operation and Maintenance Plan;
- Names of Persons or Entity Responsible for Plan Compliance;
- Construction Period Pollution Prevention Measures;
- Erosion and Sedimentation Control Plan Drawings;
- Detail drawings and specifications for erosion control BMPs, including sizing calculations;
- Vegetation Planning;
- Site Development Plan;
- Construction Sequencing Plan;
- Sequencing of Erosion and Sedimentation Controls;
- Operation and Maintenance of Erosion and Sedimentation Controls;
- Inspection Schedule;
- Maintenance Schedule;
- Inspection and Maintenance Log Form.

☐ A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan containing the information set forth above has been included in the Stormwater Report.

Checklist for Stormwater Report

Checklist (continued)

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control (continued)

☐ The project is highly complex and information is included in the Stormwater Report that explains why it is not possible to submit the Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan with the application. A Construction Period Pollution Prevention and Erosion and Sedimentation Control has ***not*** been included in the Stormwater Report but will be submitted ***before*** land disturbance begins.

☒ The project is ***not*** covered by a NPDES Construction General Permit.

☐ The project is covered by a NPDES Construction General Permit and a copy of the SWPPP is in the Stormwater Report.

☐ The project is covered by a NPDES Construction General Permit but no SWPPP been submitted. The SWPPP will be submitted BEFORE land disturbance begins.

Standard 9: Operation and Maintenance Plan

☒ The Post Construction Operation and Maintenance Plan is included in the Stormwater Report and includes the following information:

- ☒ Name of the stormwater management system owners;
- ☒ Party responsible for operation and maintenance;
- ☒ Schedule for implementation of routine and non-routine maintenance tasks;
- ☒ Plan showing the location of all stormwater BMPs maintenance access areas;
- ☐ Description and delineation of public safety features;
- ☒ Estimated operation and maintenance budget; and
- ☒ Operation and Maintenance Log Form.

☐ The responsible party is ***not*** the owner of the parcel where the BMP is located and the Stormwater Report includes the following submissions:

- ☐ A copy of the legal instrument (deed, homeowner's association, utility trust or other legal entity) that establishes the terms of and legal responsibility for the operation and maintenance of the project site stormwater BMPs;
- ☐ A plan and easement deed that allows site access for the legal entity to operate and maintain BMP functions.

Standard 10: Prohibition of Illicit Discharges

☐ The Long-Term Pollution Prevention Plan includes measures to prevent illicit discharges;

☒ An Illicit Discharge Compliance Statement is attached;

☐ NO Illicit Discharge Compliance Statement is attached but will be submitted ***prior to*** the discharge of any stormwater to post-construction BMPs.

Pipe Sizing (Culvert) Calculations

McCarty Engineering, Inc. Project: 42 Park Street Proj. No: 42 Park

Culvert Flows Date: 9/1/23

City: Ayer Comp: JLL

State: MA Check : BRM

*Shaded columns indicate input values

	Paved		Unpaved						
Culvert ID	AREA (sq. ft)	C FACTOR	AREA (sq. ft)	C FACTOR	COMPOSITE C	TOTAL AREA (sq. ft)	TOTAL AREA (acres)	Q	Double Grate
CB 1	1413	0.9	2105	0.3	0.54	3518	0.08	0.3	NO
CB 2	6486	0.9	421	0.3	0.86	6907	0.16	0.8	NO
RL 1	1427	0.9	0	0.3	0.90	1427	0.03	0.2	NO
RL 2	1427	0.9	0	0.3	0.90	1427	0.03	0.2	NO

Pipe Design Worksheet

Project # /Name: 42 Park Street

Calculated By: JLL **Date:** 9/1/2023
Checked By: BRM **Date:** 9/1/2023

**Flows form the 25-year storm event from HydroCAD were u*

n= 0.01 **HDPE Pipe**

Culvert	Q	Qsum	Length	Slope	Dia.	Full-Flow Velocity	Full-Flow Capacity[2]	
(ID, Lot #)	(cfs)	(cfs)	(ft.)	(ft./ft.)	(in.)	(fps)	(cfs)	
CB 1 to DMH 1	0.30		7.5	0.01	12	5.91	4.64	O.K
RL 1 to RL 2	0.20		161.4	0.02	8	6.38	2.23	O.K
RL 2 to DMH 1	0.40	0.40	70.0	0.01	8	4.51	1.58	O.K
DMH 1 to DMH 2	0.70	0.70	14.6	0.01	12	5.91	4.64	O.K
CB 2 to DMH 2	0.40		5.3	0.01	12	5.91	4.64	O.K
OCS 1 to FES 1*	0.19		10.0	0.01	8	4.51	1.58	O.K

[1] $V=1.49/n \times R^{2/3} \times S^{1/2}$

[2] $Q=VA$

Illicit Discharge Compliance Statement

42 Park Street
Ayer, Massachusetts

Illicit Discharge Compliance Statement

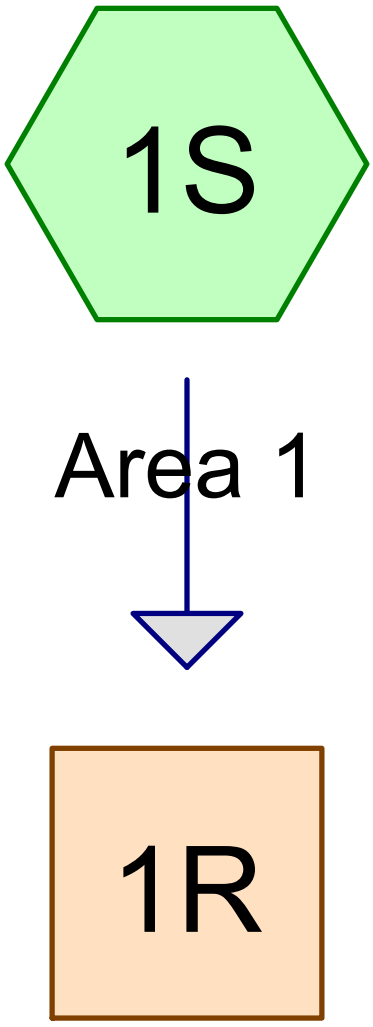
The Stormwater Management System associated with the development of 42 Park Street has been designed such that prior to storm water runoff discharging from the site, it is treated through a series of best management practices. To the Engineer's knowledge, there are no known or designed non-storm water discharges that are or will be connected to the storm water collection system that would convey pollutants directly to groundwater or surface waters.

Name: Brian Marchetti, P.E. Title: Vice President

Signature:  Date: 9/1/2023

Appendix B

Existing Conditions HydroCAD Model



Rail Road

Subcat Reach Pond Link

Routing Diagram for 2023-08-14 Existing Drainage
Prepared by McCarty Engineering, Printed 9/2/2023

Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
0.341	39	>75% Grass cover, Good, HSG A (1S)
0.077	98	Paved parking, HSG A (1S)
0.035	98	Roofs, HSG A (1S)
0.053	30	Woods, Good, HSG A (1S)
0.506	**51**	**TOTAL AREA**

Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.506	HSG A	1S
0.000	HSG B	
0.000	HSG C	
0.000	HSG D	
0.000	Other	
0.506		**TOTAL AREA**

Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.341	0.000	0.000	0.000	0.000	0.341	>75% Grass cover, Good	1S
0.077	0.000	0.000	0.000	0.000	0.077	Paved parking	1S
0.035	0.000	0.000	0.000	0.000	0.035	Roofs	1S
0.053	0.000	0.000	0.000	0.000	0.053	Woods, Good	1S
0.506	**0.000**	**0.000**	**0.000**	**0.000**	**0.506**	**TOTAL AREA**	

Time span=0.00-25.00 hrs, dt=0.01 hrs, 2501 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Area 1 Runoff Area=22,063 sf 22.12% Impervious Runoff Depth=0.11"
Flow Length=197' Tc=8.5 min CN=51 Runoff=0.01 cfs 0.005 af

Reach 1R: Rail Road Inflow=0.01 cfs 0.005 af
Outflow=0.01 cfs 0.005 af

Total Runoff Area = 0.506 ac Runoff Volume = 0.005 af Average Runoff Depth = 0.11"
77.88% Pervious = 0.394 ac 22.12% Impervious = 0.112 ac

Summary for Subcatchment 1S: Area 1

Runoff = 0.01 cfs @ 12.53 hrs, Volume= 0.005 af, Depth= 0.11"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-25.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-Year Rainfall=3.02"

Area (sf)	CN	Description
1,515	98	Roofs, HSG A
3,365	98	Paved parking, HSG A
14,861	39	>75% Grass cover, Good, HSG A
2,322	30	Woods, Good, HSG A
22,063	51	Weighted Average
17,183		77.88% Pervious Area
4,880		22.12% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.6	50	0.1000	0.13		**Sheet Flow,** Woods: Light underbrush n= 0.400 P2= 3.10"
1.5	129	0.0400	1.40		**Shallow Concentrated Flow,** Short Grass Pasture Kv= 7.0 fps
0.4	18	0.0200	0.71		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
8.5	197	Total			

Summary for Reach 1R: Rail Road

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.506 ac, 22.12% Impervious, Inflow Depth = 0.11" for 2-Year event
Inflow = 0.01 cfs @ 12.53 hrs, Volume= 0.005 af
Outflow = 0.01 cfs @ 12.53 hrs, Volume= 0.005 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-25.00 hrs, dt= 0.01 hrs

Time span=0.00-25.00 hrs, dt=0.01 hrs, 2501 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Area 1 Runoff Area=22,063 sf 22.12% Impervious Runoff Depth=0.54"
Flow Length=197' Tc=8.5 min CN=51 Runoff=0.16 cfs 0.023 af

Reach 1R: Rail Road Inflow=0.16 cfs 0.023 af
Outflow=0.16 cfs 0.023 af

Total Runoff Area = 0.506 ac Runoff Volume = 0.023 af Average Runoff Depth = 0.54"
77.88% Pervious = 0.394 ac 22.12% Impervious = 0.112 ac

Summary for Subcatchment 1S: Area 1

Runoff = 0.16 cfs @ 12.18 hrs, Volume= 0.023 af, Depth= 0.54"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-25.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-Year Rainfall=4.48"

Area (sf)	CN	Description
1,515	98	Roofs, HSG A
3,365	98	Paved parking, HSG A
14,861	39	>75% Grass cover, Good, HSG A
2,322	30	Woods, Good, HSG A
22,063	51	Weighted Average
17,183		77.88% Pervious Area
4,880		22.12% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.6	50	0.1000	0.13		**Sheet Flow,** Woods: Light underbrush n= 0.400 P2= 3.10"
1.5	129	0.0400	1.40		**Shallow Concentrated Flow,** Short Grass Pasture Kv= 7.0 fps
0.4	18	0.0200	0.71		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
8.5	197	Total			

Summary for Reach 1R: Rail Road

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.506 ac, 22.12% Impervious, Inflow Depth = 0.54" for 10-Year event
Inflow = 0.16 cfs @ 12.18 hrs, Volume= 0.023 af
Outflow = 0.16 cfs @ 12.18 hrs, Volume= 0.023 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-25.00 hrs, dt= 0.01 hrs

Time span=0.00-25.00 hrs, dt=0.01 hrs, 2501 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Area 1 Runoff Area=22,063 sf 22.12% Impervious Runoff Depth=1.02"
Flow Length=197' Tc=8.5 min CN=51 Runoff=0.42 cfs 0.043 af

Reach 1R: Rail Road Inflow=0.42 cfs 0.043 af
Outflow=0.42 cfs 0.043 af

Total Runoff Area = 0.506 ac Runoff Volume = 0.043 af Average Runoff Depth = 1.02"
77.88% Pervious = 0.394 ac 22.12% Impervious = 0.112 ac

Summary for Subcatchment 1S: Area 1

Runoff = 0.42 cfs @ 12.15 hrs, Volume= 0.043 af, Depth= 1.02"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-25.00 hrs, dt= 0.01 hrs
Type III 24-hr 25-Year Rainfall=5.61"

Area (sf)	CN	Description
1,515	98	Roofs, HSG A
3,365	98	Paved parking, HSG A
14,861	39	>75% Grass cover, Good, HSG A
2,322	30	Woods, Good, HSG A
22,063	51	Weighted Average
17,183		77.88% Pervious Area
4,880		22.12% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.6	50	0.1000	0.13		**Sheet Flow,** Woods: Light underbrush n= 0.400 P2= 3.10"
1.5	129	0.0400	1.40		**Shallow Concentrated Flow,** Short Grass Pasture Kv= 7.0 fps
0.4	18	0.0200	0.71		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
8.5	197	Total			

Summary for Reach 1R: Rail Road

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.506 ac, 22.12% Impervious, Inflow Depth = 1.02" for 25-Year event
Inflow = 0.42 cfs @ 12.15 hrs, Volume= 0.043 af
Outflow = 0.42 cfs @ 12.15 hrs, Volume= 0.043 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-25.00 hrs, dt= 0.01 hrs

Time span=0.00-25.00 hrs, dt=0.01 hrs, 2501 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Area 1 Runoff Area=22,063 sf 22.12% Impervious Runoff Depth=2.29"
Flow Length=197' Tc=8.5 min CN=51 Runoff=1.15 cfs 0.097 af

Reach 1R: Rail Road Inflow=1.15 cfs 0.097 af
Outflow=1.15 cfs 0.097 af

Total Runoff Area = 0.506 ac Runoff Volume = 0.097 af Average Runoff Depth = 2.29"
77.88% Pervious = 0.394 ac 22.12% Impervious = 0.112 ac

Summary for Subcatchment 1S: Area 1

Runoff = 1.15 cfs @ 12.13 hrs, Volume= 0.097 af, Depth= 2.29"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-25.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-Year Rainfall=7.89"

Area (sf)	CN	Description
1,515	98	Roofs, HSG A
3,365	98	Paved parking, HSG A
14,861	39	>75% Grass cover, Good, HSG A
2,322	30	Woods, Good, HSG A
22,063	51	Weighted Average
17,183		77.88% Pervious Area
4,880		22.12% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.6	50	0.1000	0.13		**Sheet Flow,** Woods: Light underbrush n= 0.400 P2= 3.10"
1.5	129	0.0400	1.40		**Shallow Concentrated Flow,** Short Grass Pasture Kv= 7.0 fps
0.4	18	0.0200	0.71		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
8.5	197	Total			

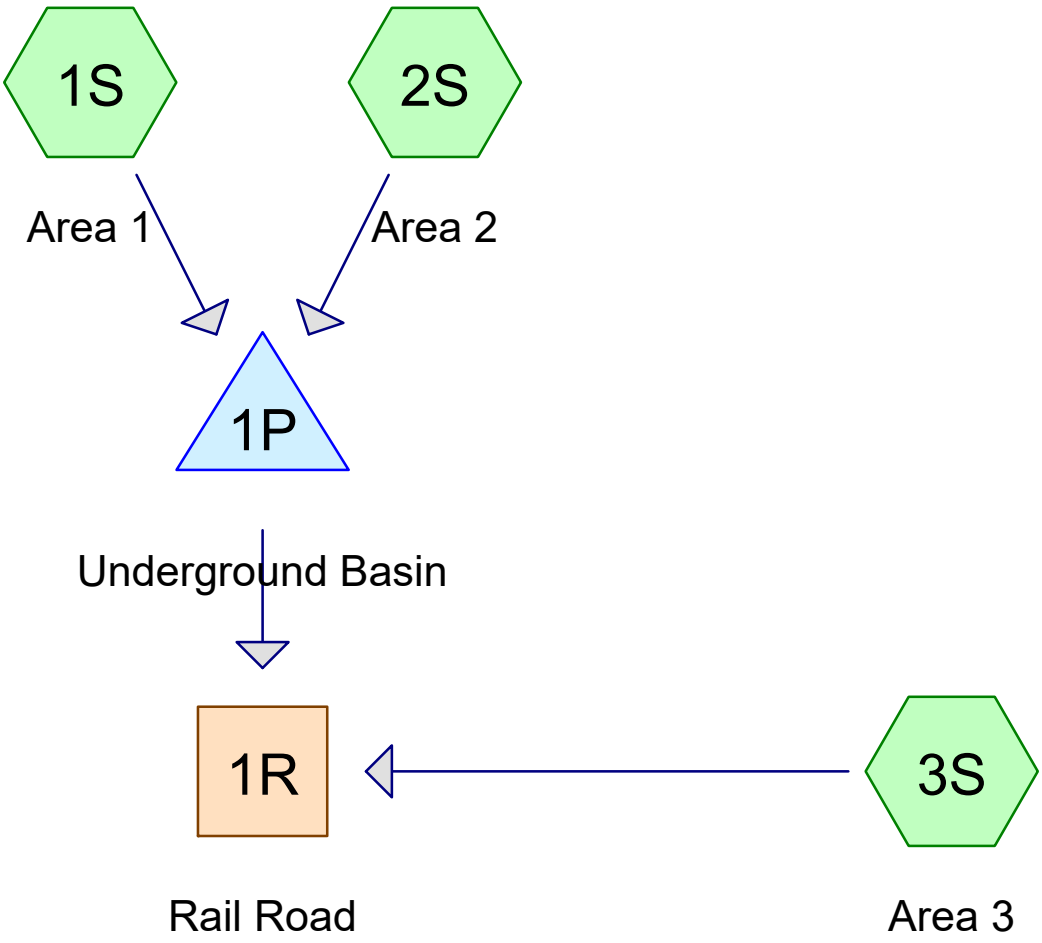
Summary for Reach 1R: Rail Road

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.506 ac, 22.12% Impervious, Inflow Depth = 2.29" for 100-Year event
Inflow = 1.15 cfs @ 12.13 hrs, Volume= 0.097 af
Outflow = 1.15 cfs @ 12.13 hrs, Volume= 0.097 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-25.00 hrs, dt= 0.01 hrs

Proposed Conditions HydroCAD Model



Subcat Reach Pond Link

Routing Diagram for 2023-08-14 Proposed Drainage
Prepared by McCarty Engineering, Printed 9/2/2023

Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
0.206	39	>75% Grass cover, Good, HSG A (2S, 3S)
0.029	98	Concrete, HSG A (2S)
0.153	98	Paved parking, HSG A (2S)
0.082	98	Roofs, HSG A (1S, 2S)
0.036	30	Woods, Good, HSG A (3S)
0.506	**69**	**TOTAL AREA**

Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.506	HSG A	1S, 2S, 3S
0.000	HSG B	
0.000	HSG C	
0.000	HSG D	
0.000	Other	
0.506		**TOTAL AREA**

Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.206	0.000	0.000	0.000	0.000	0.206	>75% Grass cover, Good	2S, 3S
0.029	0.000	0.000	0.000	0.000	0.029	Concrete	2S
0.153	0.000	0.000	0.000	0.000	0.153	Paved parking	2S
0.082	0.000	0.000	0.000	0.000	0.082	Roofs	1S, 2S
0.036	0.000	0.000	0.000	0.000	0.036	Woods, Good	3S
0.506	**0.000**	**0.000**	**0.000**	**0.000**	**0.506**	**TOTAL AREA**	

Pipe Listing (all nodes)

Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Diam/Width (inches)	Height (inches)	Inside-Fill (inches)
1	1P	214.00	213.90	10.0	0.0100	0.012	6.0	0.0	0.0

Time span=0.00-25.00 hrs, dt=0.01 hrs, 2501 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Area 1 Runoff Area=2,854 sf 100.00% Impervious Runoff Depth=2.79"
Tc=5.0 min CN=98 Runoff=0.20 cfs 0.015 af

Subcatchment 2S: Area 2 Runoff Area=11,137 sf 77.54% Impervious Runoff Depth=1.61"
Tc=5.0 min CN=85 Runoff=0.50 cfs 0.034 af

Subcatchment 3S: Area 3 Runoff Area=8,072 sf 0.00% Impervious Runoff Depth=0.00"
Tc=5.0 min CN=37 Runoff=0.00 cfs 0.000 af

Reach 1R: Rail Road Inflow=0.00 cfs 0.000 af
Outflow=0.00 cfs 0.000 af

Pond 1P: Underground Basin Peak Elev=213.05' Storage=711 cf Inflow=0.70 cfs 0.049 af
Discarded=0.10 cfs 0.049 af Primary=0.00 cfs 0.000 af Outflow=0.10 cfs 0.049 af

Total Runoff Area = 0.506 ac Runoff Volume = 0.049 af Average Runoff Depth = 1.17"
47.92% Pervious = 0.243 ac 52.08% Impervious = 0.264 ac

Summary for Subcatchment 1S: Area 1

Runoff = 0.20 cfs @ 12.07 hrs, Volume= 0.015 af, Depth= 2.79"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-25.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-Year Rainfall=3.02"

Area (sf)	CN	Description
2,854	98	Roofs, HSG A
2,854		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry,**

Summary for Subcatchment 2S: Area 2

Runoff = 0.50 cfs @ 12.08 hrs, Volume= 0.034 af, Depth= 1.61"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-25.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-Year Rainfall=3.02"

	Area (sf)	CN	Description
	720	98	Roofs, HSG A
	6,672	98	Paved parking, HSG A
*	1,244	98	Concrete, HSG A
	2,501	39	>75% Grass cover, Good, HSG A
	11,137	85	Weighted Average
	2,501		22.46% Pervious Area
	8,636		77.54% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry,**

Summary for Subcatchment 3S: Area 3

[45] Hint: Runoff=Zero

Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-25.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-Year Rainfall=3.02"

Area (sf)	CN	Description
6,491	39	>75% Grass cover, Good, HSG A
1,581	30	Woods, Good, HSG A
8,072	37	Weighted Average
8,072		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry,**

Summary for Reach 1R: Rail Road

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.506 ac, 52.08% Impervious, Inflow Depth = 0.00" for 2-Year event
Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af
Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-25.00 hrs, dt= 0.01 hrs

Summary for Pond 1P: Underground Basin

Inflow Area = 0.321 ac, 82.12% Impervious, Inflow Depth = 1.85" for 2-Year event
Inflow = 0.70 cfs @ 12.07 hrs, Volume= 0.049 af
Outflow = 0.10 cfs @ 12.60 hrs, Volume= 0.049 af, Atten= 86%, Lag= 31.8 min
Discarded = 0.10 cfs @ 12.60 hrs, Volume= 0.049 af
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-25.00 hrs, dt= 0.01 hrs
Peak Elev= 213.05' @ 12.60 hrs Surf.Area= 1,115 sf Storage= 711 cf

Plug-Flow detention time= 58.8 min calculated for 0.049 af (100% of inflow)
Center-of-Mass det. time= 58.8 min (864.6 - 805.9)

Volume	Invert	Avail.Storage	Storage Description
#1A	212.00'	1,047 cf	**34.75'W x 32.10'L x 3.50'H Field A** 3,904 cf Overall - 1,286 cf Embedded = 2,617 cf x 40.0% Voids
#2A	212.50'	1,286 cf	**ADS_StormTech SC-740 +Cap** x 28 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 28 Chambers in 7 Rows
		2,333 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	212.00'	**2.410 in/hr Exfiltration over Surface area** Conductivity to Groundwater Elevation = 210.00'
#2	Primary	214.00'	**6.0" Round Culvert** L= 10.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 214.00' / 213.90' S= 0.0100 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.20 sf

Discarded OutFlow Max=0.10 cfs @ 12.60 hrs HW=213.05' (Free Discharge)
↑**1=Exfiltration** (Controls 0.10 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=212.00' (Free Discharge)
↑**2=Culvert** (Controls 0.00 cfs)

Pond 1P: Underground Basin - Chamber Wizard Field A

Chamber Model = ADS_StormTech SC-740 +Cap (ADS StormTech® SC-740 with cap length)
Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap

51.0" Wide + 6.0" Spacing = 57.0" C-C Row Spacing

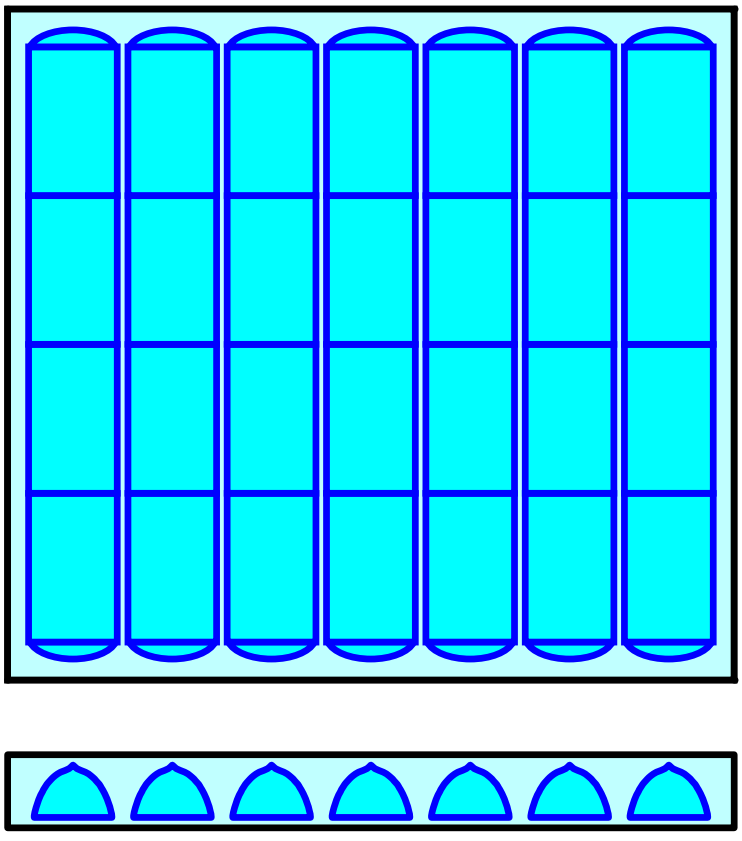
4 Chambers/Row x 7.12' Long +0.81' Cap Length x 2 = 30.10' Row Length +12.0" End Stone x 2 = 32.10' Base Length
7 Rows x 51.0" Wide + 6.0" Spacing x 6 + 12.0" Side Stone x 2 = 34.75' Base Width
6.0" Base + 30.0" Chamber Height + 6.0" Cover = 3.50' Field Height

28 Chambers x 45.9 cf = 1,286.3 cf Chamber Storage

3,903.8 cf Field - 1,286.3 cf Chambers = 2,617.4 cf Stone x 40.0% Voids = 1,047.0 cf Stone Storage

Chamber Storage + Stone Storage = 2,333.3 cf = 0.054 af
Overall Storage Efficiency = 59.8%
Overall System Size = 32.10' x 34.75' x 3.50'

28 Chambers
144.6 cy Field
96.9 cy Stone



Time span=0.00-25.00 hrs, dt=0.01 hrs, 2501 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Area 1 Runoff Area=2,854 sf 100.00% Impervious Runoff Depth=4.24"
Tc=5.0 min CN=98 Runoff=0.30 cfs 0.023 af

Subcatchment 2S: Area 2 Runoff Area=11,137 sf 77.54% Impervious Runoff Depth=2.89"
Tc=5.0 min CN=85 Runoff=0.89 cfs 0.062 af

Subcatchment 3S: Area 3 Runoff Area=8,072 sf 0.00% Impervious Runoff Depth=0.06"
Tc=5.0 min CN=37 Runoff=0.00 cfs 0.001 af

Reach 1R: Rail Road Inflow=0.00 cfs 0.001 af
Outflow=0.00 cfs 0.001 af

Pond 1P: Underground Basin Peak Elev=213.92' Storage=1,420 cf Inflow=1.19 cfs 0.085 af
Discarded=0.12 cfs 0.085 af Primary=0.00 cfs 0.000 af Outflow=0.12 cfs 0.085 af

Total Runoff Area = 0.506 ac Runoff Volume = 0.086 af Average Runoff Depth = 2.03"
47.92% Pervious = 0.243 ac 52.08% Impervious = 0.264 ac

Summary for Subcatchment 1S: Area 1

Runoff = 0.30 cfs @ 12.07 hrs, Volume= 0.023 af, Depth= 4.24"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-25.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-Year Rainfall=4.48"

Area (sf)	CN	Description
2,854	98	Roofs, HSG A
2,854		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry,**

Summary for Subcatchment 2S: Area 2

Runoff = 0.89 cfs @ 12.07 hrs, Volume= 0.062 af, Depth= 2.89"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-25.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-Year Rainfall=4.48"

	Area (sf)	CN	Description
	720	98	Roofs, HSG A
	6,672	98	Paved parking, HSG A
*	1,244	98	Concrete, HSG A
	2,501	39	>75% Grass cover, Good, HSG A
	11,137	85	Weighted Average
	2,501		22.46% Pervious Area
	8,636		77.54% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry,**

Summary for Subcatchment 3S: Area 3

Runoff = 0.00 cfs @ 15.29 hrs, Volume= 0.001 af, Depth= 0.06"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-25.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-Year Rainfall=4.48"

Area (sf)	CN	Description
6,491	39	>75% Grass cover, Good, HSG A
1,581	30	Woods, Good, HSG A
8,072	37	Weighted Average
8,072		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry,**

Summary for Reach 1R: Rail Road

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.506 ac, 52.08% Impervious, Inflow Depth = 0.02" for 10-Year event
Inflow = 0.00 cfs @ 15.29 hrs, Volume= 0.001 af
Outflow = 0.00 cfs @ 15.29 hrs, Volume= 0.001 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-25.00 hrs, dt= 0.01 hrs

Summary for Pond 1P: Underground Basin

Inflow Area = 0.321 ac, 82.12% Impervious, Inflow Depth = 3.17" for 10-Year event
Inflow = 1.19 cfs @ 12.07 hrs, Volume= 0.085 af
Outflow = 0.12 cfs @ 12.84 hrs, Volume= 0.085 af, Atten= 90%, Lag= 46.1 min
Discarded = 0.12 cfs @ 12.84 hrs, Volume= 0.085 af
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-25.00 hrs, dt= 0.01 hrs
Peak Elev= 213.92' @ 12.84 hrs Surf.Area= 1,115 sf Storage= 1,420 cf

Plug-Flow detention time= 106.5 min calculated for 0.085 af (100% of inflow)
Center-of-Mass det. time= 106.5 min (900.5 - 794.0)

Volume	Invert	Avail.Storage	Storage Description
#1A	212.00'	1,047 cf	**34.75'W x 32.10'L x 3.50'H Field A** 3,904 cf Overall - 1,286 cf Embedded = 2,617 cf x 40.0% Voids
#2A	212.50'	1,286 cf	**ADS_StormTech SC-740 +Cap** x 28 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 28 Chambers in 7 Rows
		2,333 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	212.00'	**2.410 in/hr Exfiltration over Surface area** Conductivity to Groundwater Elevation = 210.00'
#2	Primary	214.00'	**6.0" Round Culvert** L= 10.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 214.00' / 213.90' S= 0.0100 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.20 sf

Discarded OutFlow Max=0.12 cfs @ 12.84 hrs HW=213.92' (Free Discharge)
↑**1=Exfiltration** (Controls 0.12 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=212.00' (Free Discharge)
↑**2=Culvert** (Controls 0.00 cfs)

Pond 1P: Underground Basin - Chamber Wizard Field A

Chamber Model = ADS_StormTech SC-740 +Cap (ADS StormTech® SC-740 with cap length)
Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap

51.0" Wide + 6.0" Spacing = 57.0" C-C Row Spacing

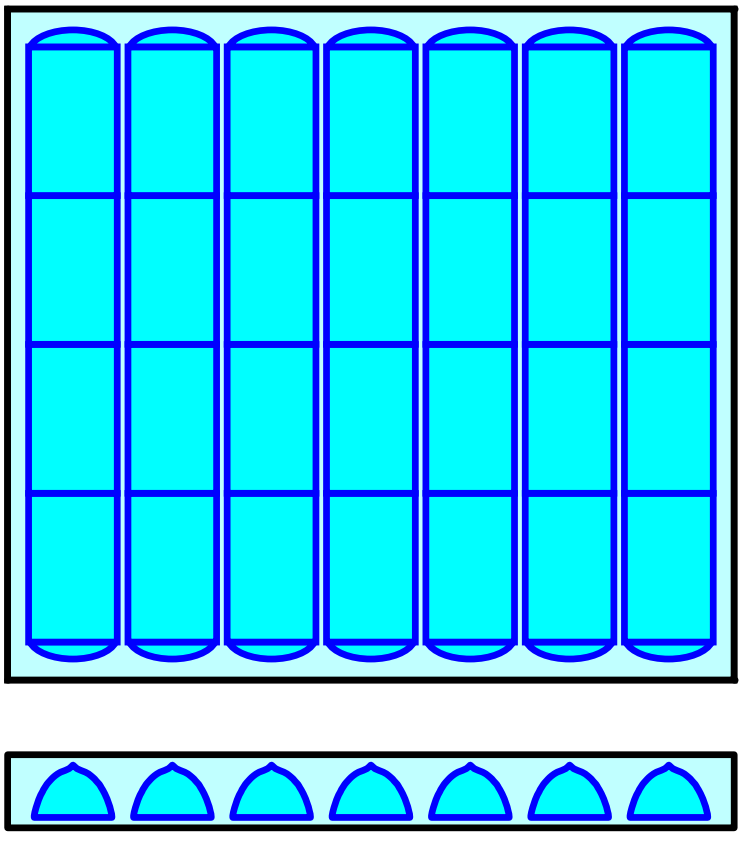
4 Chambers/Row x 7.12' Long +0.81' Cap Length x 2 = 30.10' Row Length +12.0" End Stone x 2 = 32.10' Base Length
7 Rows x 51.0" Wide + 6.0" Spacing x 6 + 12.0" Side Stone x 2 = 34.75' Base Width
6.0" Base + 30.0" Chamber Height + 6.0" Cover = 3.50' Field Height

28 Chambers x 45.9 cf = 1,286.3 cf Chamber Storage

3,903.8 cf Field - 1,286.3 cf Chambers = 2,617.4 cf Stone x 40.0% Voids = 1,047.0 cf Stone Storage

Chamber Storage + Stone Storage = 2,333.3 cf = 0.054 af
Overall Storage Efficiency = 59.8%
Overall System Size = 32.10' x 34.75' x 3.50'

28 Chambers
144.6 cy Field
96.9 cy Stone



Time span=0.00-25.00 hrs, dt=0.01 hrs, 2501 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Area 1 Runoff Area=2,854 sf 100.00% Impervious Runoff Depth=5.37"
Tc=5.0 min CN=98 Runoff=0.37 cfs 0.029 af

Subcatchment 2S: Area 2 Runoff Area=11,137 sf 77.54% Impervious Runoff Depth=3.94"
Tc=5.0 min CN=85 Runoff=1.20 cfs 0.084 af

Subcatchment 3S: Area 3 Runoff Area=8,072 sf 0.00% Impervious Runoff Depth=0.25"
Tc=5.0 min CN=37 Runoff=0.01 cfs 0.004 af

Reach 1R: Rail Road Inflow=0.28 cfs 0.016 af
Outflow=0.28 cfs 0.016 af

Pond 1P: Underground Basin Peak Elev=214.39' Storage=1,764 cf Inflow=1.58 cfs 0.113 af
Discarded=0.14 cfs 0.102 af Primary=0.27 cfs 0.012 af Outflow=0.40 cfs 0.113 af

Total Runoff Area = 0.506 ac Runoff Volume = 0.117 af Average Runoff Depth = 2.77"
47.92% Pervious = 0.243 ac 52.08% Impervious = 0.264 ac

Summary for Subcatchment 1S: Area 1

Runoff = 0.37 cfs @ 12.07 hrs, Volume= 0.029 af, Depth= 5.37"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-25.00 hrs, dt= 0.01 hrs
Type III 24-hr 25-Year Rainfall=5.61"

Area (sf)	CN	Description
2,854	98	Roofs, HSG A
2,854		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry,**

Summary for Subcatchment 2S: Area 2

Runoff = 1.20 cfs @ 12.07 hrs, Volume= 0.084 af, Depth= 3.94"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-25.00 hrs, dt= 0.01 hrs
Type III 24-hr 25-Year Rainfall=5.61"

	Area (sf)	CN	Description
	720	98	Roofs, HSG A
	6,672	98	Paved parking, HSG A
*	1,244	98	Concrete, HSG A
	2,501	39	>75% Grass cover, Good, HSG A
	11,137	85	Weighted Average
	2,501		22.46% Pervious Area
	8,636		77.54% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry,**

Summary for Subcatchment 3S: Area 3

Runoff = 0.01 cfs @ 12.43 hrs, Volume= 0.004 af, Depth= 0.25"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-25.00 hrs, dt= 0.01 hrs
Type III 24-hr 25-Year Rainfall=5.61"

Area (sf)	CN	Description
6,491	39	>75% Grass cover, Good, HSG A
1,581	30	Woods, Good, HSG A
8,072	37	Weighted Average
8,072		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry,**

Summary for Reach 1R: Rail Road

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.506 ac, 52.08% Impervious, Inflow Depth = 0.37" for 25-Year event
Inflow = 0.28 cfs @ 12.43 hrs, Volume= 0.016 af
Outflow = 0.28 cfs @ 12.43 hrs, Volume= 0.016 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-25.00 hrs, dt= 0.01 hrs

Summary for Pond 1P: Underground Basin

Inflow Area = 0.321 ac, 82.12% Impervious, Inflow Depth = 4.23" for 25-Year event
Inflow = 1.58 cfs @ 12.07 hrs, Volume= 0.113 af
Outflow = 0.40 cfs @ 12.43 hrs, Volume= 0.113 af, Atten= 74%, Lag= 21.8 min
Discarded = 0.14 cfs @ 12.43 hrs, Volume= 0.102 af
Primary = 0.27 cfs @ 12.43 hrs, Volume= 0.012 af

Routing by Stor-Ind method, Time Span= 0.00-25.00 hrs, dt= 0.01 hrs
Peak Elev= 214.39' @ 12.43 hrs Surf.Area= 1,115 sf Storage= 1,764 cf

Plug-Flow detention time= 105.4 min calculated for 0.113 af (100% of inflow)
Center-of-Mass det. time= 105.3 min (892.8 - 787.4)

Volume	Invert	Avail.Storage	Storage Description
#1A	212.00'	1,047 cf	**34.75'W x 32.10'L x 3.50'H Field A** 3,904 cf Overall - 1,286 cf Embedded = 2,617 cf x 40.0% Voids
#2A	212.50'	1,286 cf	**ADS_StormTech SC-740 +Cap** x 28 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 28 Chambers in 7 Rows
		2,333 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	212.00'	**2.410 in/hr Exfiltration over Surface area** Conductivity to Groundwater Elevation = 210.00'
#2	Primary	214.00'	**6.0" Round Culvert** L= 10.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 214.00' / 213.90' S= 0.0100 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.20 sf

Discarded OutFlow Max=0.14 cfs @ 12.43 hrs HW=214.39' (Free Discharge)
↑**1=Exfiltration** (Controls 0.14 cfs)

Primary OutFlow Max=0.27 cfs @ 12.43 hrs HW=214.39' (Free Discharge)
↑**2=Culvert** (Barrel Controls 0.27 cfs @ 2.25 fps)

Pond 1P: Underground Basin - Chamber Wizard Field A

Chamber Model = ADS_StormTech SC-740 +Cap (ADS StormTech® SC-740 with cap length)
Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap

51.0" Wide + 6.0" Spacing = 57.0" C-C Row Spacing

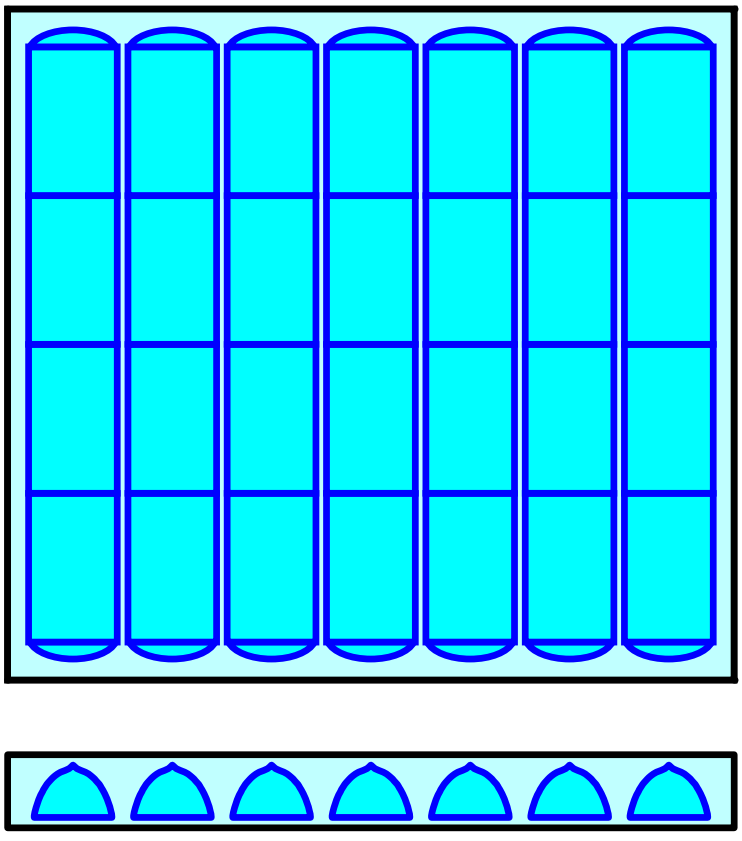
4 Chambers/Row x 7.12' Long +0.81' Cap Length x 2 = 30.10' Row Length +12.0" End Stone x 2 = 32.10' Base Length
7 Rows x 51.0" Wide + 6.0" Spacing x 6 + 12.0" Side Stone x 2 = 34.75' Base Width
6.0" Base + 30.0" Chamber Height + 6.0" Cover = 3.50' Field Height

28 Chambers x 45.9 cf = 1,286.3 cf Chamber Storage

3,903.8 cf Field - 1,286.3 cf Chambers = 2,617.4 cf Stone x 40.0% Voids = 1,047.0 cf Stone Storage

Chamber Storage + Stone Storage = 2,333.3 cf = 0.054 af
Overall Storage Efficiency = 59.8%
Overall System Size = 32.10' x 34.75' x 3.50'

28 Chambers
144.6 cy Field
96.9 cy Stone



Time span=0.00-25.00 hrs, dt=0.01 hrs, 2501 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Area 1 Runoff Area=2,854 sf 100.00% Impervious Runoff Depth=7.65"
Tc=5.0 min CN=98 Runoff=0.53 cfs 0.042 af

Subcatchment 2S: Area 2 Runoff Area=11,137 sf 77.54% Impervious Runoff Depth=6.11"
Tc=5.0 min CN=85 Runoff=1.83 cfs 0.130 af

Subcatchment 3S: Area 3 Runoff Area=8,072 sf 0.00% Impervious Runoff Depth=0.93"
Tc=5.0 min CN=37 Runoff=0.11 cfs 0.014 af

Reach 1R: Rail Road Inflow=0.89 cfs 0.061 af
Outflow=0.89 cfs 0.061 af

Pond 1P: Underground Basin Peak Elev=215.40' Storage=2,288 cf Inflow=2.36 cfs 0.172 af
Discarded=0.17 cfs 0.125 af Primary=0.80 cfs 0.047 af Outflow=0.97 cfs 0.172 af

Total Runoff Area = 0.506 ac Runoff Volume = 0.186 af Average Runoff Depth = 4.41"
47.92% Pervious = 0.243 ac 52.08% Impervious = 0.264 ac

Summary for Subcatchment 1S: Area 1

Runoff = 0.53 cfs @ 12.07 hrs, Volume= 0.042 af, Depth= 7.65"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-25.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-Year Rainfall=7.89"

Area (sf)	CN	Description
2,854	98	Roofs, HSG A
2,854		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry,**

Summary for Subcatchment 2S: Area 2

Runoff = 1.83 cfs @ 12.07 hrs, Volume= 0.130 af, Depth= 6.11"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-25.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-Year Rainfall=7.89"

	Area (sf)	CN	Description
	720	98	Roofs, HSG A
	6,672	98	Paved parking, HSG A
*	1,244	98	Concrete, HSG A
	2,501	39	>75% Grass cover, Good, HSG A
	11,137	85	Weighted Average
	2,501		22.46% Pervious Area
	8,636		77.54% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry,**

Summary for Subcatchment 3S: Area 3

Runoff = 0.11 cfs @ 12.12 hrs, Volume= 0.014 af, Depth= 0.93"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-25.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-Year Rainfall=7.89"

Area (sf)	CN	Description
6,491	39	>75% Grass cover, Good, HSG A
1,581	30	Woods, Good, HSG A
8,072	37	Weighted Average
8,072		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry,**

Summary for Reach 1R: Rail Road

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 0.506 ac, 52.08% Impervious, Inflow Depth = 1.45" for 100-Year event
Inflow = 0.89 cfs @ 12.25 hrs, Volume= 0.061 af
Outflow = 0.89 cfs @ 12.25 hrs, Volume= 0.061 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-25.00 hrs, dt= 0.01 hrs

Summary for Pond 1P: Underground Basin

Inflow Area = 0.321 ac, 82.12% Impervious, Inflow Depth = 6.42" for 100-Year event
Inflow = 2.36 cfs @ 12.07 hrs, Volume= 0.172 af
Outflow = 0.97 cfs @ 12.26 hrs, Volume= 0.172 af, Atten= 59%, Lag= 11.4 min
Discarded = 0.17 cfs @ 12.26 hrs, Volume= 0.125 af
Primary = 0.80 cfs @ 12.26 hrs, Volume= 0.047 af

Routing by Stor-Ind method, Time Span= 0.00-25.00 hrs, dt= 0.01 hrs
Peak Elev= 215.40' @ 12.26 hrs Surf.Area= 1,115 sf Storage= 2,288 cf

Plug-Flow detention time= 91.7 min calculated for 0.172 af (100% of inflow)
Center-of-Mass det. time= 91.7 min (869.7 - 778.0)

Volume	Invert	Avail.Storage	Storage Description
#1A	212.00'	1,047 cf	**34.75'W x 32.10'L x 3.50'H Field A** 3,904 cf Overall - 1,286 cf Embedded = 2,617 cf x 40.0% Voids
#2A	212.50'	1,286 cf	**ADS_StormTech SC-740 +Cap** x 28 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 28 Chambers in 7 Rows
		2,333 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	212.00'	**2.410 in/hr Exfiltration over Surface area** Conductivity to Groundwater Elevation = 210.00'
#2	Primary	214.00'	**6.0" Round Culvert** L= 10.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 214.00' / 213.90' S= 0.0100 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.20 sf

Discarded OutFlow Max=0.17 cfs @ 12.26 hrs HW=215.40' (Free Discharge)
1=Exfiltration (Controls 0.17 cfs)

Primary OutFlow Max=0.80 cfs @ 12.26 hrs HW=215.40' (Free Discharge)
2=Culvert (Inlet Controls 0.80 cfs @ 4.07 fps)

Pond 1P: Underground Basin - Chamber Wizard Field A

Chamber Model = ADS_StormTech SC-740 +Cap (ADS StormTech® SC-740 with cap length)
Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap

51.0" Wide + 6.0" Spacing = 57.0" C-C Row Spacing

4 Chambers/Row x 7.12' Long +0.81' Cap Length x 2 = 30.10' Row Length +12.0" End Stone x 2 = 32.10' Base Length
7 Rows x 51.0" Wide + 6.0" Spacing x 6 + 12.0" Side Stone x 2 = 34.75' Base Width
6.0" Base + 30.0" Chamber Height + 6.0" Cover = 3.50' Field Height

28 Chambers x 45.9 cf = 1,286.3 cf Chamber Storage

3,903.8 cf Field - 1,286.3 cf Chambers = 2,617.4 cf Stone x 40.0% Voids = 1,047.0 cf Stone Storage

Chamber Storage + Stone Storage = 2,333.3 cf = 0.054 af
Overall Storage Efficiency = 59.8%
Overall System Size = 32.10' x 34.75' x 3.50'

28 Chambers
144.6 cy Field
96.9 cy Stone